DEPTH, FEET	SAMPLE	SAMPLE NO.	PEN./TORVANE SPT_RI OU CONNT	JOB NO PROJEC LOCATI ELEVAT MANUFA GROUNE WEATHE	D. <u>11400</u> TT <u>UPPer</u> TON <u>Along</u> TION OF H ACTURER'S WATER: D WATER: D	Basin Basin Ole DESIGNAT EPTH CLO	COMPL Detartion A Bay All LION OF DRILL ft., ELEV.	RIG Marsh Buggy K4x4, F. 36 ft., at end of Drill: $MV \neq HOT 1$	<u> </u>	
- 0 -	KA			COLOR	MATERIA TYPE	CONSIS- TENCY	SECONDARY CONSTITUENTS	THE GOLUTINITY		
	R	$\frac{1}{2}$	0.75	GRAY	1	5771		-w/ROOTS 0-4 BROWN 0:	-	
- 5		23	1.5 2.0	GRAI	CLAN CLAY	STIFF UKRI STIFF		- WISOND POEKETS 4:61 - Wils 4-6		, ,
		4		Lt GRAY	CLAY	VARY	SANDY	AYELLEWW6-8		
		s	1.75	1_{L+}	CLAY	STILE	SANDY	- Willer CNode L- -Wilfe Nods L- + BROWN 8-10		
- 10-			<u>,</u> 22	GEAT REODISH	f	VERT		W SILT LAYER 10' 10'3"		
		7	1.7	BROWF REDDISH REDDISH	CLAY	STIFF		- W/Calc Nods 10'3"-13'4" VGRAY 10: 14 -W SANDSELT SKAMS 12: -W/CLAWFZIN HOLE 12.13		٠
+15-	X	9	233		<u> </u>		SZ27y -	- WICHAWFZIH HOLE 12 . 13		
		~			T		-, .,	· · · · ·	$\left[- \right]$	
		7	গস্য	L+GRAY	MAN	VERY STLFF		+ REDW AD 18-3.2	$\left - \right $	
-20-			<u>2:13</u> 271	114				+ BROW 10 18-32 -W/Cg/c Neds 18-22 -W/FE Nods 18-32	Н	
			<u><</u> 	CRAY	CLAY CLAY	VERY STIFT VRRY STZFF		-WISLICKANSIOKO		
		╈	2.7J	GRAI Lt	CLAY	VERY		-w/Calc Nods 25-32		
- 25-		1	3.0	GPAY L+		STZFF				
		+	325		CLAY	VERY STIFF		:		
-30-		1	3,25	GRAY		VERY STIFF			-	
		5 <u> </u> -	3.5	GRAY	CLAY	UKRY STIFT			-	
	170	<u> </u> /	25 	LT GRAY	CLA-1 CLAV	STIFF STIFF	JANRY	· · · · · · · · · · · · · · · · · · ·	-1	
- 35 -	<u>()</u>)	<u>1</u> //	,75	L+GPAN	CLAN	57=86	SANDY			
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U.S. ARMY CORPS OF ENGINEERS

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GEOTEST ENGINEERING, INC.

Project: Brays Bayou PDM

SUMMARY OF LABORATORY TEST RESULTS

Contract No. DACW64-95-D-0007 Delivery Order No. 0035

Boring No. 97-23

S # Depth		PP	PP	SPT Blows	Visual		Мс	Dry Unit	Wet Unit	LL	PL			anical Ar % Passin	-		Torvane Shear	qu
	(ft)	(tsf)	per Foot	Classification		(%)	Wt (pcf)	Wt (pcf)	(%)	(%)	#4	#10	#40	#100	#200	Strength (tsf)	(tsf)	
1	0 - 2	0.75		Clay,w/grass roots,Medium stiff,Gray & Brown	СН	25.7												
2	2 - 4	1.50		Clay,w/grass roots,Stiff,Gray & Brown	СН	25.7	96.3	121.0	52.0	19.0								
3	4 - 6	2.00		Clay,w/ferrous nodules,Very stiff,Gray & brown	СН	21.0	····											
4	6 - 8	2.25		Sandy Clay,w/calcareous & ferrous nodules, Very stiff,Gray	CL	19.2												
5	8 - 10	1.75		Sandy Clay,w/calcareous & ferrous nodules, Stiff,Gray	CL	19.0												
6	10 - 12	2.50		Clay,w/calcreous nodules & silt seams, slickensided,Very stiff,Reddish brown	СН	22.0												
7	12 - 14	1.75		Clay,w/calcareous nodules & sand seams, slickensided,Stiff,Reddish brown	СН	21.9												
8	14-15.5		6	Silty Sand,Loose,Reddish brown	SM													
9	18-20	2.75		Clay,w/calc & fe nod,slickensided,Very stiff, Gray & Brown	СН	21.4												
10	20-22	2.75		Clay,w/calc & fe nod,slickensided,Very stiff, Gray & Brown	СН	20.5												
11	22-24	2.75		Clay,w/caic & fe nod,slickensided,Very stiff, Gray & Brown	СН	25.0	95.2	118.9	74.0	25.0	100.0	99.1	98.7	98.2	97.1		2.31	
12	24-26	3.00		Clay,w/calc & fe nod,slickensided,Very stiff, Gray & Brown	СН	26.1												
13	26-28	3.25		Clay,w/calc & fe nod,slickensided,Very stiff, Gray & Brown	СН	25.1			<u> </u>									
14	28-30	3.25		Clay,w/calc & fe nod,slickensided,Very stiff, Gray & Brown	СН	26.6												
15	30-32	3.50		Clay,w/calc & fe nod,slickensided,Very stiff, Gray & Brown	СН	22.7												
16	32-34	3.00		Clay,w/calc & fe nod,slickensided,Very stiff, Gray & Brown	СН	18.8												
17	34-35	1.75		Sandy Clay,Stiff,Gray	CL	16.4	110.7	128.9	33.0	16.0				 		ļ	ļ	
												<u> </u>			<u> </u>			
			1		1			1									1	

S # : Sample Number, P P : Pocket Penetrometer Reading, U S C : Unified Soil Classification, M c : Moisture Content q u : Uncogined Compressive Strength, W O H : Weight of hammer, W O P : Weight of Pipe

Page 3 of 9

DATE 7/7/97 JOB NO. 114008613 PROJECT Brays Bayou PDM AREA Houston, Texas BORING NO. 97-23 SAMPLE NO. 11 DEPTH 22-24 ft SPECIMEN NO. 1 CLASSIFICATION Clay,w/calc & fer nod,slickensided,Very stiff,Gray & Brown 5.595 in. W2 Height Tare No. Average Diameter 500.13 gm Average Diama Initial Area 2.830 in. Tare plus Wet Specimen 6.290 sq in. Tare plus Dry Specimen 408.75 gm Volume 35.194 cu in. 91.38 gm Water Weight Volume of Solids cu in. Void Ratio Saturation % Dry Density 95.2 lb/cu ft 42.74 gm Tare Weight Wet Specimen 1098.50 gm 879.04 gm Dry Specimen Water Content 24.97 % Specific Gravity of Solids PI = 49LL = 74PL = 25Proving Ring No. 10170 Proving Ring Constant, K = .766 lbs/div.

Elapsed Time min.	Dial Reading 0.001"	Cumulative Change in.	Proving Ring Dial Reading	Axial Load lb	Axial Strain	Area Corr. sq in.	Compr. Stress tsf
.0	Ο.	.000	.0	.0	.000	6.29	.000
.2	10.	.010	50.0	38.3	.002	6.30	.438
.4	20.	.020	92.0	70.5	.004	6.31	.804
.6	30.	.030	152.0	116.4	.005	6.32	1.326
.8	40.	.040	212.0	162.4	.007	6.34	1.846
.9	50.	.050	240.0	183.8	.009	6.35	2.086
1.1	60.	.060	255.0	195.3	.011	6.36	2.212
1.4	80.	.080	263.0	201.5	.014	6.38	2.273
1.8	100.	.100	268.0	205.3	.018	6.40	2.308
2.0	120.	.120	267.0	204.5	.021	6.43	2.291

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Job No. 114008613

					EM 1	110-2-1 Appendix 30 Nov
Controlled stress	2.4 1.8 1.8 1.2 1.2 0.6 0.0					
	0.0	0.	6	1.2	1.8	
Controlled strain			Axial	Stroin, x		
Test No.	· · · ·		Axial			
Test No. Type of Specimen			Axial 1 Undisturbed	Stroin, %		2
Test No. Type of Specimen Water content		Wo	Axial			×
Test No. Type of Specimen Water content	· · · · · · · · · · · · · · · · · · ·	w ₀ e ₀	Axial 1 Undisturbed 25.0 %	Strain, %		8
Test No. Type of Specimen Water content Void ratio Soturation	·	wo eo So	Axial 1 Undisturbed 25.0 % %	Strain, %		
Test No. Type of Specimen Water content Void ratio Saturation Dry density, lb/cu	·	₩0 €0 S0 7d	Axial 	Strain, %		8
Test No. Type of Specimen Water content Void ratio Saturation		wo eo So	Axial 1 Undisturbed 25.0 % %	Strain, %		8
Test No. Type of Specimen Water content Void ratio Soturation Dry density, lb/cu Time to failure, min Unconfined compressive	ft e	wo eo So γd tr	Axial 	Strain, %		8
Test No. Type of Specimen Water content Void ratio Saturation Dry density, lb/cu Time to failure, min Unconfined compressive strength, T/sq ft	ft e	wo eo So γd tr qu	Axial 1 Undisturbed 25.0 % % 95.2 1.78 2.31	Strain, %		8
Test No. Type of Specimen Water content Void ratio Soturation Dry density, lb/cu Time to failure, min Unconfined compressive strength, T/sq ft Undrained shear strengt	ft e gth, T/sq ft	Ψο e₀ S₀ γ₀ tr qu S₀	Axial 1 Undisturbed 25.0 % % 95.2 1.78 2.31	Strain, %		8
Test No. Type of Specimen Water content Void ratio Saturation Dry density, Ib/cu Time to failure, min Unconfined compressive strength, T/sq ft Undrained shear streng Sensitivity ratio	ft e gth, T/sq ft ter, in.	wo eo So γd tr qu Su St	Axial 1 Undisturbed 25.0 % % 95.2 1.78 2.31 1.15	Strain, %		8
Test No. Type of Specimen Water content Void ratio Saturation Dry density, lb/cu Time to failure, min Unconfined compressive strength, T/sq ft Undrained shear streng Sensitivity ratio Initial specimen diamet	ft e gth, T/sq ft ter, in.	wo eo So γd tr qu Su Su Do Ho	Axial 1 Undisturbed 25.0 % 95.2 1.78 2.31 1.15 2.830 5.595	Strain, %		8
Test No. Type of Specimen Water content Void ratio Soturation Dry density, lb/cu Time to failure, min Unconfined compressive strength, T/sq ft Undrained shear streng Sensitivity ratio Initial specimen diamet	ft e gth, T/sq ft ter, in. , in.	wo eo So γd tr qu Su Su Do Ho	Axial 1 Undisturbed 25.0 % 95.2 1.78 2.31 1.15 2.830 5.595	Strain, %	G ₈	8
Test No. Type of Specimen Water content Void ratio Saturation Dry density, lb/cu Time to failure, min Unconfined compressive strength, T/sq ft Undrained shear streng Sensitivity ratio Initial specimen diamet Initial specimen height, Classification Clay,w/c	ft e gth, T/sq ft ter, in. , in.	wo eo So γd tr qu Su Su Ho	Axial 1 Undisturbed 25.0 % 95.2 1.78 2.31 1.15 2.830 5.595 ery stiff,Groy & Pl	Strain, %		8
Test No. Type of Specimen Water content Void ratio Saturation Dry density, lb/cu Time to failure, min Unconfined compressive strength, T/sq ft Undrained shear streng Sensitivity ratio Initial specimen diamet Initial specimen height, Classification Clay,w/c LL 74	ft e gth, T/sq ft ter, in. , in.	wo eo So Yd tr qu Su Su Su Boo Ho ensided,Ve 25	Axial 1 Undisturbed 25.0 % 95.2 1.78 2.31 1.15 2.830 5.595 ery stiff,Groy & Pl	Strain, %		8
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Test No. Type of Specimen Water content Void ratio Saturation Dry density, lb/cu Time to failure, min Unconfined compressive strength, T/sq ft Undrained shear streng Sensitivity ratio Initial specimen diamet Initial specimen height, Classification Clay,w/c LL 74	ft e gth, T/sq ft ter, in. , in.	wo eo So 7d tr qu Su Su Su Su Su Su Su Su Su Su Su Su Su	Axial 1 Undisturbed 25.0 s 95.2 1.78 2.31 1.15 2.830 5.595 ery stiff,Groy & Pl et Brays Bayo Houston, Texo No. 97–23	Strain, %	Ga Somple No.	% % %

– Geotest Engineering, Inc. –