

Appendix B contains discussions of analytical procedures used in our engineering analyses. Appendix C contains a positioning report by Fugro Chance, Inc., of Lafayette, Louisiana.

For the purposes of discussion and presentation, "driven pipe pile" is used in this report to represent foundation piles, caissons and conductors, unless otherwise specified.

# 2.2 FIELD AND LABORATORY INVESTIGATIONS

The field investigation was performed on June 26 through 28, 2008, from the R/V Seaprobe. The soil conditions were determined by performing four exploratory borings, two at each SPM location with one boring at a selected anchor leg location, and one boring at the proposed PLET location. Enterprise Field Services selected the boring locations. These borings were drilled to a penetration of 131-ft below mudline. The water depths at the boring locations ranged from 110 to 113 ft. A chronological summary of field operations is presented in Appendix A.

## 2.2.1 Exploratory Borings

FMMG personnel drilled the soil borings with a DMX drill rig positioned over the centerwell of the R/V Seaprobe. The vessel was anchored at the boring location by a 4-point mooring system. Soil conditions at the site were explored by drilling a group of four soil borings to 131-ft penetration below the seafloor. The final coordinates for the boring locations are presented in Table 2-1. A plan of borings within Block A-36, of the Galveston Area is presented on Plate 2-1. Fugro Chance, Inc., of Lafayette, Louisiana, conducted surveying utilizing STARFIX and DGPS, and performed a 360-degree scanning sonar survey. The positioning report, prepared by Fugro Chance, is presented in Appendix C. The scanning sonar reports are available from Fugro Chance upon request.

**Table 2-1: Final Boring Coordinates** (Texas South Central Zone Coordinates)

FMMG Boring Designation	Fugro Chance Boring Designation	Proposed Boring Coordinates	Final Boring Coordinates	Boring Termination Depth (ft)
SPM #1 PLET	Core 3	X = 3,276,605.26 ft Y = 265,296.65 ft	X = 3,276,615 ft Y = 265,270 ft	131
SPM #1 Anchor Leg #2	Core 1	X = 3,275,201.70 ft Y = 264,859.70 ft	X = 3,275,180 ft Y = 264,853 ft	131
SPM #2 PLET	Core 4	X = 3,283,609.94 ft Y = 269,139.12 ft	X = 3,283,617 ft Y = 269,118 ft	131
SPM #2 Anchor Leg #6	Core 2	X = 3,284,713.02 ft Y = 270,110.77 ft	X = 3,284,733 ft Y = 270,117 ft	131

Samples were obtained through 5.0-in.-OD, 4.5-in.-IF drill pipe at all the locations. Samples were spaced at 3-ft intervals to 20-ft penetration, at 5-ft intervals to 68-ft penetration, and at 10-ft intervals thereafter to the final boring depth at all the locations, except at the SPM #2 PLET location. Sampling intervals at the SPM #2 PLET location was completed as follows: 3-ft intervals to 23-ft penetration, 5-ft intervals to 71-ft penetration, and 10-ft intervals thereafter to the final boring depth. Additionally, a 5-ft





shallow boring, designated as Core 4A by Fugro Chance, was drilled at the SPM #2 PLET location to allow re-sampling. The drilling and sampling techniques used to complete these borings are explained in detail in Appendix A.

Two water depths were measured at each boring location using a seafloor sensor seated in the drill bit. The water depth measurements are tabulated in Table 2-2. These water depth measurements are intended for the purpose of the geotechnical investigation only, and are not corrected for tidal or other variations. If utilized for other purposes, the water depth measurement should be adjusted to account for meteorological tide and datum corrections. The water depth measuring procedures are explained in detail in Appendix A.

Boring Designation	Water Depth (ft)	Time and Date of Measurement	Supplemental Water Depth (ft)	Time and Date of Measurement
SPM #1 PLET	112	1630 hours on June 27, 2008	113	2245 hours on June 27, 2008
SPM #1 ANCHOR LEG #2	113	2400 hours on June 26, 2008	113	0605 hours on June 27, 2008
SPM #2 PLET	112	0250 hours on June 28, 2008	112	1030 hours on June 28, 2008
SPM #2 ANCHOR LEG #6	110	1450 hours on June 28, 2008	111	2105 hours on June 28, 2008

**Table 2-2: Measured Water Depths** 

### 2.2.2 Field and Laboratory Tests

The soil testing program was designed to evaluate pertinent index and engineering properties of the foundation soils. During the field operation, all samples were extruded from the sampler and classified by the soil technician or field engineer. Unit weight, Torvane, pocket penetrometer, miniature vane and unconsolidated-undrained triaxial compression tests were performed in the field on selected cohesive samples. All of the samples were shipped to Fugro's Houston laboratory where Atterberg limit tests, water content tests, and grain-size analyses, as well as additional density tests, unconsolidated-undrained triaxial compression tests, and miniature vane tests, were performed.

A description of relevant laboratory procedures is provided in Appendix A. The strength and classification test results are presented graphically on the Logs of Boring and Test Results in Section 3. Grain-size distribution curves from sieve-analyses and stress-strain curves from triaxial compression tests are presented in Appendix A.

## 2.3 GENERAL SOIL CONDITIONS

### 2.3.1 Soil Stratigraphy

The soil stratigraphy at each of the boring locations disclosed by the field and laboratory investigation is presented in Section 3. The soil stratigraphy is based on the classification of soil samples



2-3



### 3.4 SPM #2 ANCHOR LEG #6 LOCATION

#### 3,4.1 Introduction

The field investigation at the location designated as SPM #2 ANCHOR LEG #6 was performed on June 28, 2008. Soil sampling was performed to 131-ft penetration at Texas South Central Zone Coordinates X = 3,284,733 ft and Y = 270,117 ft. The measured water depth ranged from 110 to 111 ft.

### 3.4.2 Soil Stratigraphy

The soil stratigraphy disclosed by the field and laboratory investigations is presented on the boring log, Plate 3-34. The soil stratigraphy is based on the classification of soil samples recovered from the boring and observations made during drilling operations. A generalized summary of the major soil strata is tabulated below.

	Penetra <sup>*</sup>	tion, ft	
<u>Stratum</u>	From	<u>To</u>	<u>Description</u>
ı	0	5	Very soft to soft lean clay
il	5	11	Firm lean clay
íll	11	28	Loose to medium dense silt with sand to sandy silt
IV ·	28	38	Firm lean clay
V	38	58	Loose to medium dense silt
VI	58	131	Firm to very stiff clay

Detailed soil descriptions that include textural variations and inclusions are noted on the boring log. A key to the terms and symbols used on the boring log is presented on Plate 2-2. The Roman numeral representing each stratum is also shown on the boring log and on relevant plates. The variation in soil stratigraphy across this site is indicated in a comparison (integration) of the geophysical and geotechnical soil information presented on Plate 3-35.

### 3.4.2.1 Interpretation of Soil Properties

The shear strength and submerged unit weight profiles shown on Plates 3-36 and 3-37, respectively, best represent the assembled test results plotted on the boring log. These profiles were used in the engineering analyses.

### 3.4.3 Pile Design Information

The pile design information developed for this study includes ultimate axial capacities, axial load-pile movement data, and lateral soil resistance-pile deflection (p-y) characteristics. The analytical methods used to develop this information are presented briefly in Section 2.5 and in more detail in Appendix B.

## 3.4.3.1 Axial Pile Design

**Ultimate Axial Capacity.** The unit skin friction and unit end bearing values plotted on Plates 3-38 and 3-39, respectively, was calculated using the API RP 2A methods described in Appendix B. These values were used to calculate the ultimate axial compressive and tensile capacities for 42-in.-diameter pipe piles, driven to final penetration at the boring location. Capacity curves for driven pipe piles (conductors, caissons, anchor and foundation piles) are presented on Plate 3-40.





API RP 2A recommends that pile penetrations be selected using appropriate factors of safety or pile resistance factors. These factors are discussed in Section 2.5.1 of this report.

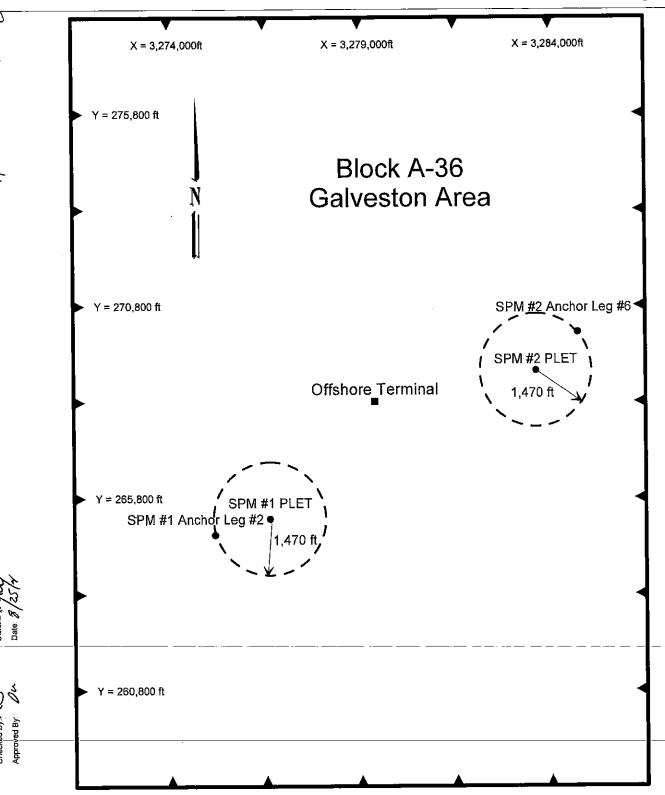
**Axial Load Transfer Data**. Axial load-pile movement analyses are usually performed using a computer solution based on methods developed by Reese (1964) or Matlock, et al. (1976). Plates 3-41 and 3-42 present the results as side load-side movement (t-z) and tip load-tip movement (Q-z) data for 42-in.-diameter driven pipe piles, respectively. The Q-z data should be used for foundation piles and neglected for caissons and conductor design. In developing the axial load transfer data in the cohesive soils, a post-peak adhesion ratio of 0.90 was utilized.

## 3.4.3.2 Lateral Pile Design Data

The soil resistance-pile deflection (p-y) characteristics of the soils at the boring location were developed for individual 42-in.-diameter driven pipe piles. These data may be used in lateral load analyses of driven piles, conductors and caissons. The p-y data for cyclic loading were developed to 100-ft penetration using procedures that have been outlined in API RP 2A and briefly explained in Appendix B. The stratigraphy and parameters used to develop the p-y data are presented on Plate 3-43. The p-y data for 42-in.-diameter driven pipe piles are presented on Plate 3-44. P-y values presented at 100-ft penetration may be used for lateral load analyses at greater depths.





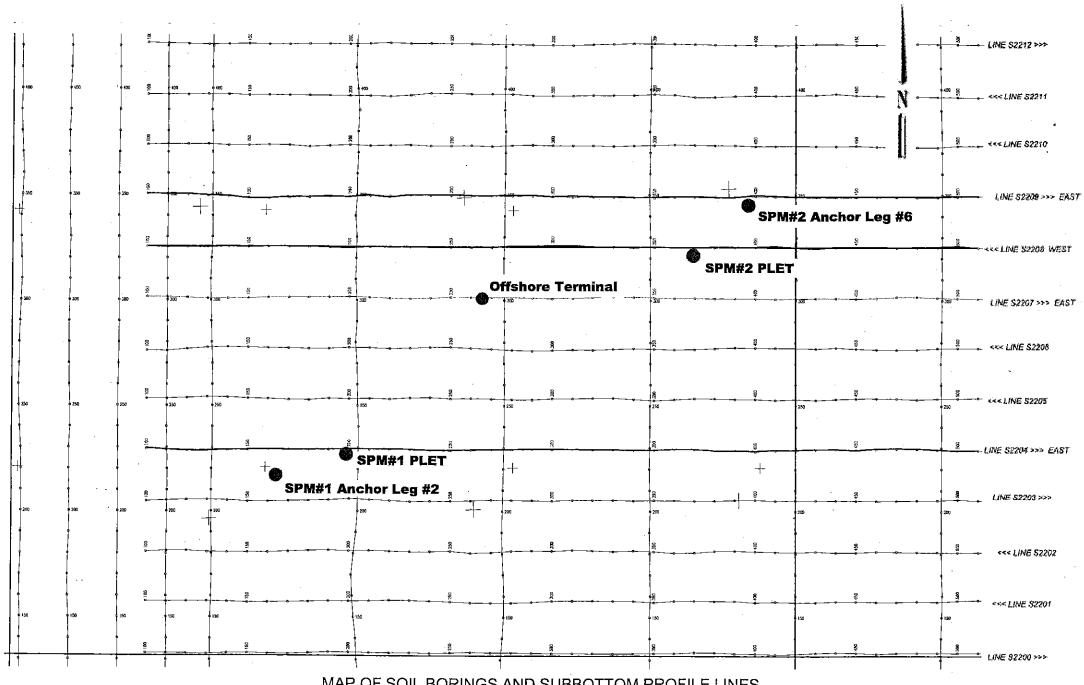


Projection: Texas South Central Zone Coordinates

# **PLAN OF BORINGS**

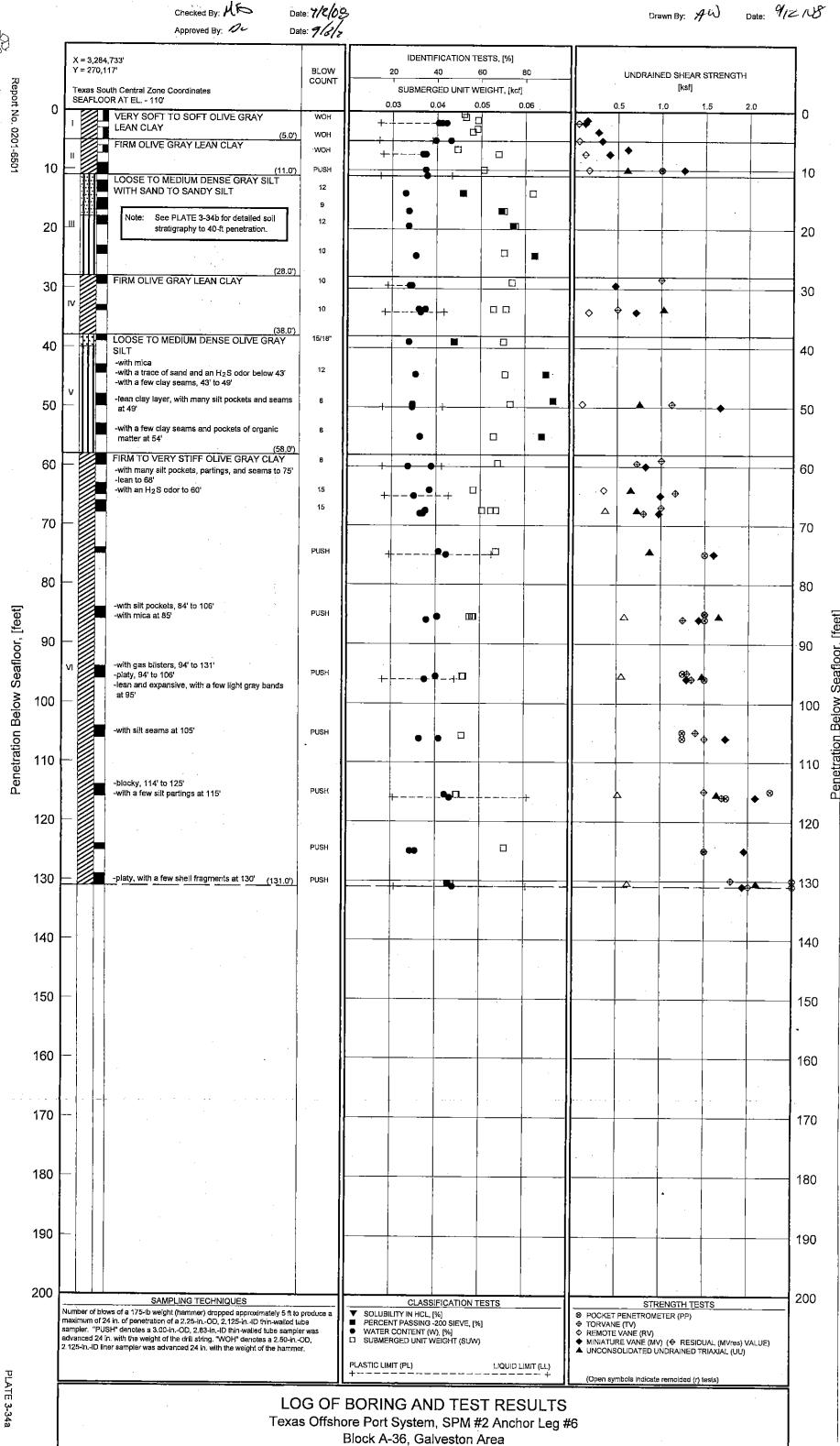
Texas Offshore Port System, Offshore Terminal Location Block A-36, Galveston Area



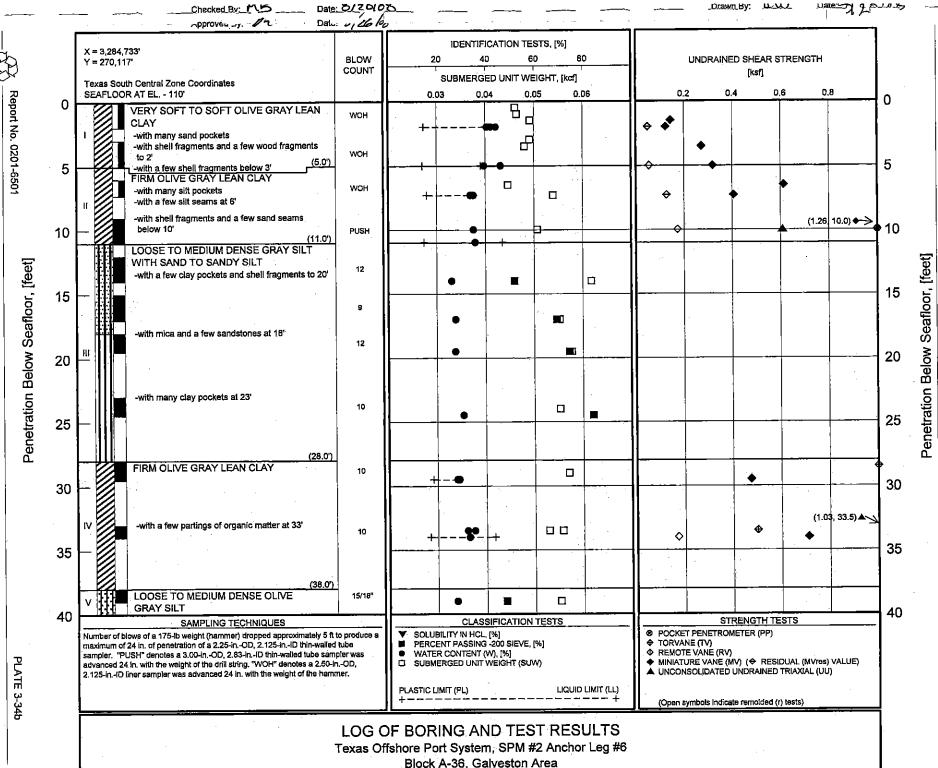


MAP OF SOIL BORINGS AND SUBBOTTOM PROFILE LINES

Texas Offshore Port System Block A-36, Galveston Area









Drawn By: PANA

GEOPHYSICAL AND GEOTECHNICAL INTEGRATION Texas Offshore Port System, SPM #2 Anchor Leg #6 Block A36, Galveston Area

Report No. 0201-6501

CLASSIFICATION TESTS

\*\* SOCUSICITY IN HOL. [15]

\*\* FERCENT PASSING -000 SIEVE [15]

\*\* WATER CONTENT WAY, [14]

\*\* SUBMERGED UNIT WEIGHT (SUM)

STRENOTH TESTS

POCKET PRINTEROMETER (PP)

TORMANE (PT)

REMOTE WASE (NV)

REMOTE WASE (NV)

REMOTE WASE (NV)

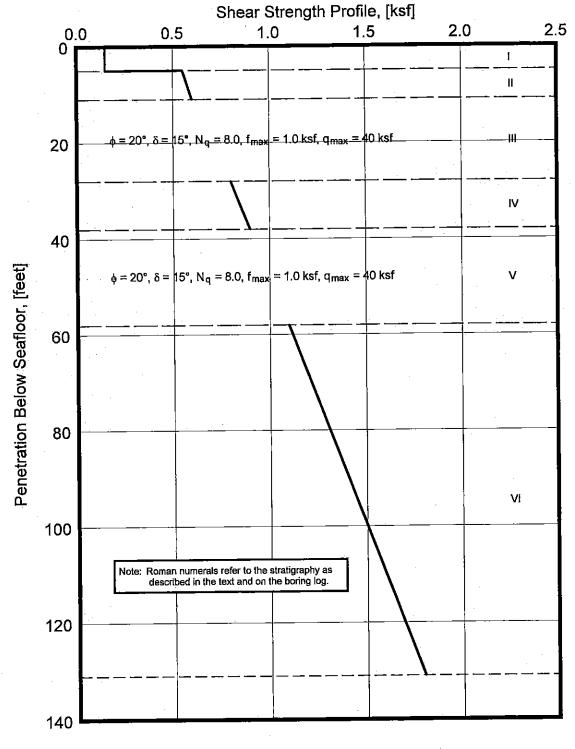
REMOTE WASE (NV)

MICHAEL (NV)

PLATE 3-35

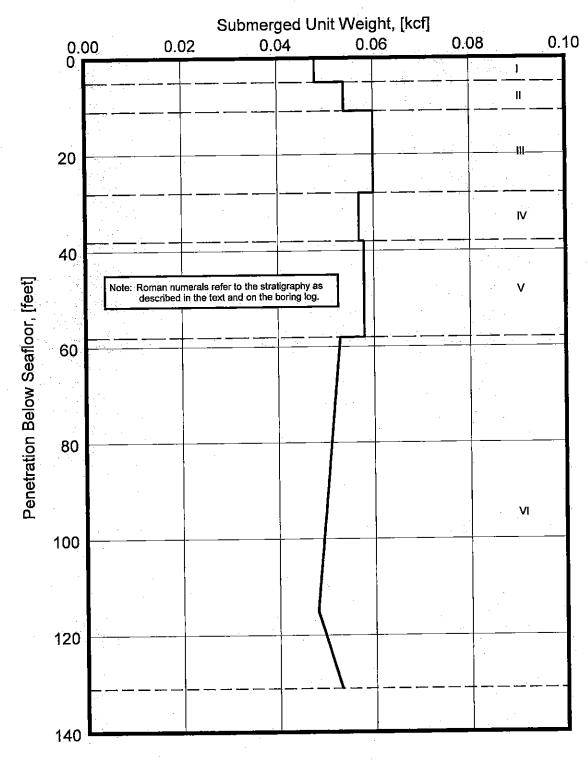
Portion of Subbottom Profiler

Line 2207



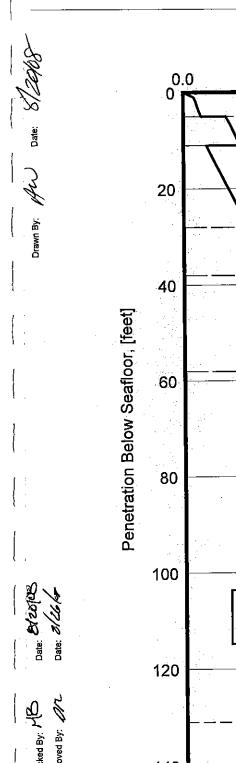
# **DESIGN STRENGTH PARAMETERS**

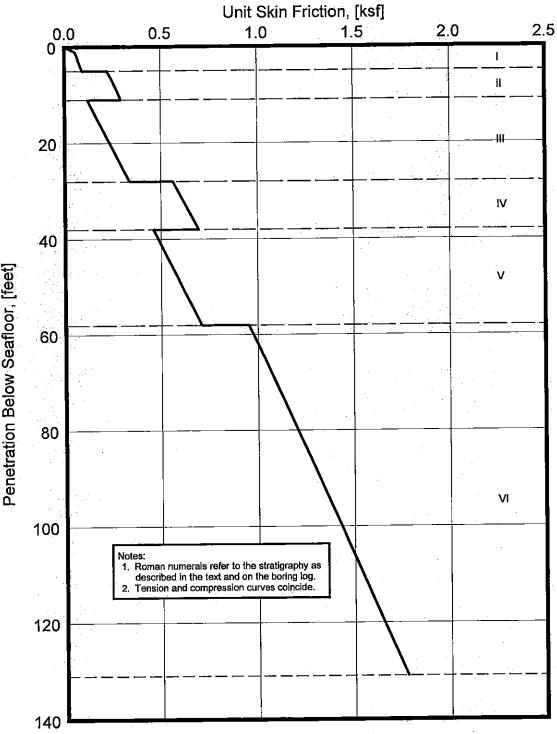




# DESIGN SUBMERGED UNIT WEIGHT

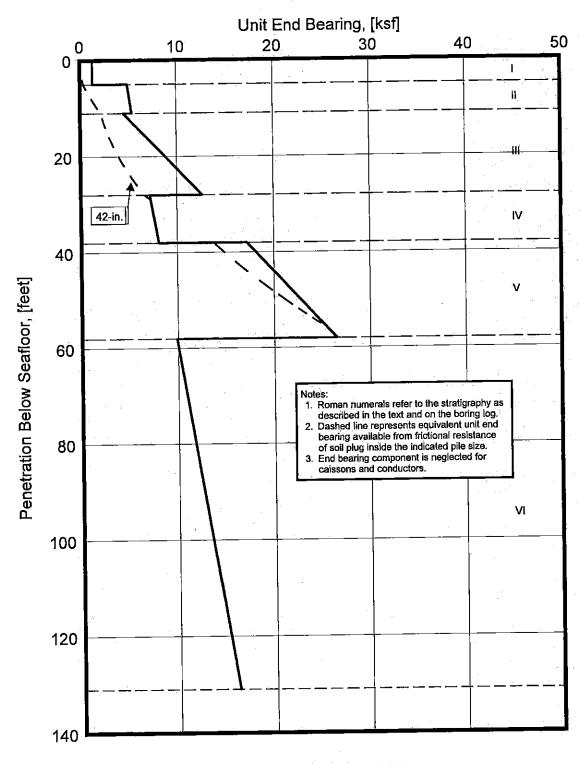




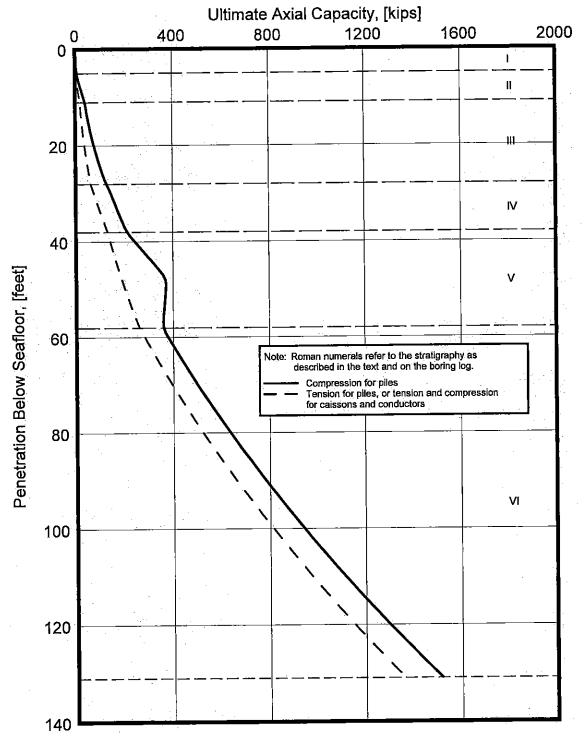


UNIT SKIN FRICTION API RP 2A (2000) Method

Checked By: RC



UNIT END BEARING API RP 2A (2000) Method



# **ULTIMATE AXIAL CAPACITY**

API RP 2A (2000) Method 42-in.-Diameter Driven Pipe Piles Texas Offshore Port System, SPM #2 Anchor Leg #6 Block A-36, Galveston Area



PENETRATION BELOW MUDLINE		<u> </u>	<u>.</u>		CURVE PO	INTS			
(feet)		11	2	3	4	5	6	7	8
0.0	t	0.00	0.00 0.07	0.00 0.13	0.00 0.24	0.00 0.34	0.00 0.42	0.00 0.84	0.00 42.00
1.0	t	0.00 0.00	0.02 0.07	0.03 0.13	0.04 0.24	0.05 0.34	0.06 0.42	0.05 0.84	0.05 42.00
3.0	t z	0.00	0.02 0.07	0.04 0.13	0.06 0.24	0,07 0,34	0.07 0.42	0.07 0.84	0.07 42.00
5.0	t	0.00 0.00	0.03 0.07	0,05 0.13	0.07 0.24	0.09 0.34	0.09 0.42	0.09 0.84	0.09 42.00
5.0	t	0.00	0.07 0.07	0.11 0.13	0.17 0.24	0.20 0.34	0.22 0.42	0.20 0.84	0.20 42.00
11.0	t	0.00	0.09 0.07	0.15 0.13	0.22 0.24	0.27 0.34	0.30 0.42	0.27 0.84	0.27 42.00
11.0	t	0.00	0.12 0.10	0.12 42.00					
28.0	t z	0.00	0.34 0.10	0.34 42.00					
28.0	t	0.00 0.00	0.17 0.07	0.28 0.13	0.42 0.24	0.51 0.34	0.56 0.42	0.51 0.84	0.51 42.00
38.0	t	0.00 0.00	0.21 0.07	0.35 0.13	0.52 0.24	0.63 0.34	0.70 0.42	0.63 0.84	0.63 42.00
38.0	t	0.00	0.46 0.10	0.46 42.00	· · ·				
58.0	t	0.00	0.71 0.10	0.71 42.00	· ·		· · ·		
58.0	t	0.00	0.29 0.07	0.48 0.13	0.72 0.24	0.86 0.34	0.95 0.42	0.86 0.84	0.86 42.00
115.0	t	0.00 0.00	0.48 0.07	0.80 0.13	1.20 0.24	1.44 0.34	1.60 0.42	1.44 0.84	1.44 42.00
131.0	t	0.00 0.00	0.53 0.07	0.89 0.13	1.33 0.24	1.60 0.34	1.77 0.42	1.60 0.84	1.60 42.00
		!							

Notes: 1. "t" is mobilized soil-pile adhesion, [ksf].
2. "z" is axial pile displacement, [in.].
3. Data for tension and compression coincide.

# **AXIAL LOAD TRANSFER DATA**

(T-Z DATA)

API RP 2A (2000) Method

42-in -Diameter Driven Pipe Piles

Texas Offshore Port System, SPM #2 Anchor Leg #6

Block A-36, Galveston Area

Report No. 0201-6501

PENETRATION BELOW MUDLINE			<del>-</del>	CL	JRVE POINTS				-
(feet)		1	2	3	4	5	6	7	
58.0	Q	0 0.00	24 0,08	48 0.55	71 1.76	86 3.07	95 4.20	95 42.00	
131.0	Q z	0 0.00	39 0.08	78 0.55	117 1.76	140 3.07	156 4.20	156 42.00	
							•		
					·				
			·						
		•							
									. *
							<u>·.                                    </u>		

Notes: 1. "Q" is mobilized end bearing capacity, [kips]. 2. "z" is axial tip displacement, [in.].

# **AXIAL LOAD TRANSFER DATA**

(Q-Z DATA)

API RP 2A (2000) Method

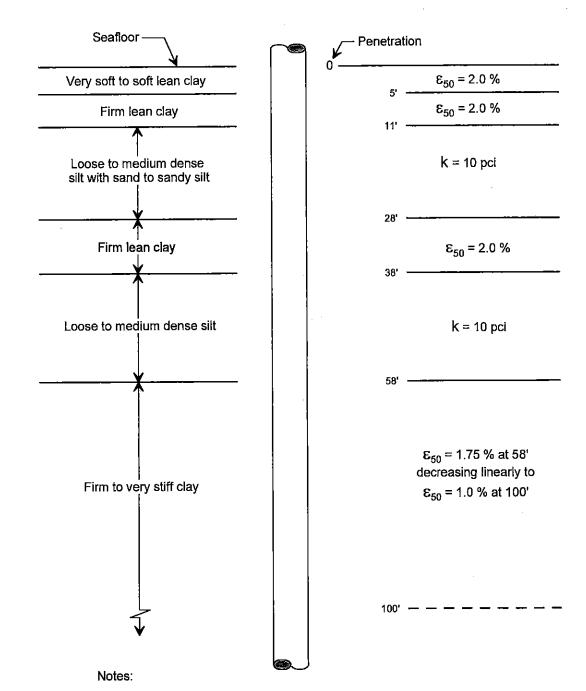
42-in.-Diameter Driven Pipe Piles

Texas Offshore Port System, SPM #2 Anchor Leg #6

Block A-36, Galveston Area







- 1.  $\mathcal{E}_{50}$  is axial strain at half of peak deviator stress for cohesive soils.
- 2. Soil strength parameters are shown on Plate 3-36,
- 3. Submerged unit weight profile is shown on Plate 3-37.
- 4. k is the modulus of horizontal subgrade reaction for granular soils.

# STRATIGRAPHY AND PARAMETERS FOR P-Y DATA



TU	GRO
	_~_
	=>=
	<u> </u>

PENETRATION BELOW MUDLINE					CURVE PO	DINTS			
(feet)		1	2	3	4	5	6	7	8
0.0	p y	0 0,00	20 0.06	30 0.21	45 0.63	66 2.10	94 6.30	0 31.50	0 42.00
3.0	P y	0 0.00	29 0.06	44 0.21	65 0.63	96 2.10	138 6.30	32 31.50	32 42.00
5.0	p y	0.00	35 0.06	53 0.21	79 0.63	116 2.10	167 6.30	65 31.50	65 42.00
5.0	p y	0.00	100 0.06	153 0.21	226 0.63	333 2.10	479 6.30	92 31.50	92 42.00
8.0	p y	0 0.00	122 0.06	187 0.21	276 0.63	406 2.10	585 6.30	179 31.50	179 42.00
11.0	p y	0.00	145 0.06	222 0.21	328 0.63	482 2.10	694 6.30	291 31.50	291 42.00
11.0	p y	0 0.00	174 0.14	291 0.24	384 0.35	482 0.52	552 0.81	575 1.17	581 42.00
15.0	p y	0 0.00	303 0.17	505 0.31	667 0.45	839 0.67	960 1.03	1001 1.49	1011 42.00
23.0	p y	0 0.00	659 0.25	1099 0.44	1450 0.63	1824 0.95	2087 1.46	2175 2.11	2197 42.00
28.0	p y	0.00	948 0.29	1580 0,52	2085 0.75	2622 1.12	3002 1.72	3128 2.49	3160 42.00
28.0	p y	0.00	314 0.06	482 0.21	712 0.63	1048 2.10	1509 6.30	1504 31.50	1504 42.00
29.0	p y	0.00	319 0.06	489 0.21	723 0.63	1063 2.10	1531 6.30	1531 42.00	
38.0	p y	0 0.00	354 0.06	543 0.21	803 0.63	1181 2.10	1701 6.30	1701 42.00	
38.0	p y	0.00	1459 0.33	2431 0.59	3209 0.85	4036 1.27	4620 1.95	4814 2.82	4863 42.00
58.0	p y	0 0.00	2244 0.33	3741 0.59	4938 0.85	6210 1.28	7107 1.97	7407 2.84	7481 42.00
58.0	p y	0.00	433 0.05	664 0.18	982 0,55	1444 1.84	2079 5.51	2079 42.00	
100.0 (and below)	p y	0.00	592 0.03	907 0.10	1341 0.31	1973 1.05	2841 3.15	2841 42.00	
							÷		

Notes: 1. "p" is soil resistance, [lb/in.]. 2. "y" is lateral deflection, [in.].

# P-Y DATA

(CYCLIC LOADING)

API RP 2A (2000) Method

42-in.-Diameter Driven Pipe Piles

Texas Offshore Port System, SPM #2 Anchor Leg #6

Block A-36, Galveston Area



Checked By: UB Approved By: AL

Date: 9/2/08

Drawn By: Tomol

Date: 9/2/08

# **Summary of Test Results**

Job No.: 0201-6501-4

02-Sep-2008 (Ver. #6)

Boring: Texas Offshore Port System, SPM #2 Anchor Leg #6

Block: A-36

Area: Galveston

				Identifi	cation T	ests		Strength E		Miniat	re Vane (ksf)	Tests				Con	pressio	n Test	s		
Sample No.	Depth (ft)	Liquidity Index	Liquid Limit (%)	Plastic Limit (%)	Moisture Content (%)	Submerged Unit Weight (pcf)	Passing No. 200 Sieve (%)	Penetrometer		Undisturbed		Residual	Type Test	Moisture Content (%)	Confining Pressure (psi)	Undisturbed Strength (ksf)	Remolded Strength (ksf)	€ 50 Strain (%)	Submerged Unit Weight (pcf)	Failure Strain (%)	Type of Failure
1	0.50					46															
2	1.00					46										p)					
3	1.50					49				0.14											
4	2.00	1.09	40	15	42																
4	2.00				41						0.05										
4	2.00				44					0.12		0.05								-	-
5	3.00					49														-	-
6	3.50					48				0.27											-
7	5.00										0.06						-		<b></b>		
7	5.00	1.06	38	14	39									-				-	-	-	—
7	5.00				46					0.32			_					-	ļ	-	-
8	6.50					45				0.61							-	-	ļ		
9	7.30				35	54				0.41		0.13	_					-		<del> </del>	₩
9	7.30	.93	35	16	34						<b>.</b>				-			-		-	-
10	10.00								1		0.17			<u> </u>	ļ		-		<u> </u>	1	<del>                                     </del>
10	10.00							1.00	1.00	1.26			UU	35	121	0.61		2.1	51	16	A
11	10.50													-		-	-	-		-	+-
12	11.00	.65	47	15	36								_	-		-	-	-		+	+
13	14.00				26	62	52						-	-		-	-	-	-	+	-
14	17.00				28	55	69						-	-	-	-	-	+	-	-	+
15	19.50				27	58	74						-	-		-	-	-	-	+-	+
16	24.00	0		14		55				<u> </u>						Plus Signs		<u> </u>	<u> </u>		<u></u>

U - Unconfined Compression

UU- Unconsolidated-Undrained Triaxial

CU- Consolidated-Undrained Triaxial

A - Bulge

B - Single Shear Plane

C - Multiple Shear Plane

D - Vertical Fracture

Plus Signs [+] denote tests which exceeded the capacity of the measuring device.

NP = Non Plastic Material



**Summary of Test Results** 

Date: 9/2/08 Date: 9/2/08 Drawn By: Tlonol Date: 9/2/08

Job No.: 0201-6501-4

02-Sep-2008 (Ver. #6)

Boring: Texas Offshore Port System, SPM #2 Anchor Leg #6

Block: A-36

Area: Galveston

				Identifi	ication 1	Tests		Strength I		Miniati	ure Vane	Tests				Con	npressio	n Test	s		
Sample No.	Depth (ft)	Liquidity Index	Liquid Limit (%)	Plastic Limit (%)	Moisture Content (%)	Submerged Unit Weight (pcf)	Passing No. 200 Sieve (%)	(ks	T) Torvane	Undisturbed	(ksf)	Residual	Type Test	Moisture Content (%)	Confining Pressure (psi)	Undisturbed Strength (ksf)	Remolded Strength (ksf)	E 50 Strain (%)	Submerged Unit Weight (pcf)	Failure Strain (%)	Type of Failure
17	24.50				31		84														
117	28.50								1.00												
18	29.00					57															
19	29.50				28					0.47										<u> </u>	
19	29.50	1.00	29	18	29																
20	33.50				35	53			0.50				υυ	32	120	1.03		5.6	56	20	AB
21	34.00	.61	43	17	33					0.71											
21	34.00										0.17								20		
22	39.00				28	55	48														
23	44.50				31	56	89													_	
24	49.00						93														
25	49.50			() ()							0.10										
25	49.50					57			1.12				UU	29	121	0.75		3.5	57	21	Α
26	50.00	.49	43	16	29					1.67										1	
27	55.00				33	53	88														
28	59.00								1.00											1	
29	59.50					54			0.72											ļ	
30	60.00	.43	42	16	27													_		ļ	
30	60.00				38					0.82								_		1	<del> </del>
31	64.00										0.35										
31	64.00					48							UU	37	120	0.65		1.8	48	19	A
32	64.50								1.16												

NOTES:

TYPE OF TEST

U - Unconfined Compression

UU- Unconsolidated-Undrained Triaxial

CU- Consolidated-Undrained Triaxial

TYPE OF FAILURE

A - Bulge

B - Single Shear Plane

C - Multiple Shear Plane

D - Vertical Fracture

Plus Signs [+] denote tests which exceeded the capacity of the measuring device.

NP = Non Plastic Material



Report No. 0201-6501

Report No. 0201-6501

# **Summary of Test Results**

Job No.: 0201-6501-4

02-Sep-2008 (Ver. #6)

Boring: Texas Offshore Port System, SPM #2 Anchor Leg #6

Block: A-36

Area: Galveston

				Identif	ication 1	Γests		Strength I		Miniati	ure Vane	Tests				Con	npressio	n Test	ts		
Sample No.	Depth (ft)	Liquidity Index	Liquid Limit (%)	Plastic Limit (%)	Moisture Content (%)	Submerged Unit Weight (pcf)	Passing No. 200 Sieve (%)	(ks	Torvane	Undisturbed	(ksf)	Residual	Type Test	Moisture Content (%)	Confining Pressure (psi)	Undisturbed Strength (ksf)	Remolded Strength (ksf)	Eso Strain (%)	Submerged Unit Weight (pcf)	Failure Strain (%)	Type of Failure
33	65.00	.46	46	17	30					0.99											
34	67.00								1.00												
35	67.50												UU	35	120	0.73		2.0	52	16	А
35	67.50					54							บบ	35	120		0.37		50		
36	68.00				33																
36	68.00				34				0.80	0.98											
37	74.50					54							บบ	41	120	0.87		2.2	54	10	А
38	75.00	.56	65	19	45			1.50	1.60	1.60											
39	85.00							1.50	1.50												
40	85.50					48							υυ	41	120	1.66		1.1	49	4	AB
40	85.50												υυ	41	120		0.59		48		
41	86.00				36			1.50	1.25	1.44											
42	95.00							1.25	1.30												
43	95.50												υυ		121		0.55	1	46		<u> </u>
43	95.50												UU	40	121	1.47		0.9	46	4	В
44	96.00	.58	49	16	35			1.50	1.35	1.30								_			_
45	105.00							1.25	1.40									<u> </u>			
46	105.50					46							_					_			1
47	106.00				42			1.25	1.50	1.74								_	ļ		↓
47	106.00				33													-			-
48	115.00							2.25	1.50									-	1	-	-
49	115.50												UU		121		0.52		45		

NOTES:

TYPE OF TEST

U - Unconfined Compression

UU- Unconsolidated-Undrained Triaxial

CU- Consolidated-Undrained Triaxial

TYPE OF FAILURE

A - Bulge

B - Single Shear Plane

C - Multiple Shear Plane

D - Vertical Fracture

Plus Signs [+] denote tests which exceeded the capacity of the measuring device.

NP = Non Plastic Material



Checked By: 48

Date: 9/2/08 Date: 9/2/08

# **Summary of Test Results**

Job No.: 0201-6501-4

02-Sep-2008 (Ver. #6)

Boring: Texas Offshore Port System, SPM #2 Anchor Leg #6

Block: A-36

Area: Galveston

									***************************************					100							
				Identif	ication 1	Гests		Strength E		Miniati	ure Vane	Tests				Con	npressio	n Test	ts		
Sample No.	Depth (ft)	Liquidity Index	Liquid Limit (%)	Plastic Limit (%)	Moisture Content (%)	Submerged Unit Weight (pcf)	Passing No. 200 Sieve (%)	(ks	Torvane	Undisturbed	(ksf)	Residual	Type Test	Moisture - Content (%)	Confining Pressure (psi)	Undisturbed Strength (ksf)	Remolded Strength (ksf)	E 50 Strain (%)	Submerged Unit Weight (pcf)	Failure Strain (%)	Type of Failure
49	115.50												UU	44	120	1.64		0.8	45	3	С
50	116.00	.42	81	21	46			1.75	1.70	2.08											
51	124.50					56															
52	125.00				31			1.50	1.50	1.96											<u> </u>
52	125.00				29			8													_
53	130.00							2.50	1.80												<u> </u>
54	130.50												บบ		122		0.63		44		_
54	130.50		19										υυ	46	119	2.09		1.2	43	3	В
55	131.00	.44	81	22	48			2.50	2.00	1.93											

NOTES:

TYPE OF TEST

U - Unconfined Compression

UU- Unconsolidated-Undrained Triaxial

CU- Consolidated-Undrained Triaxial

TYPE OF FAILURE

A - Bulge

B - Single Shear Plane

C - Multiple Shear Plane

D - Vertical Fracture

Plus Signs [+] denote tests which exceeded the capacity of the measuring device.

NP = Non Plastic Material

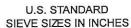


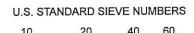
PERCENT PASSING BY WEIGHT

Checked by: AB Approved by: Date: 9/2/08 Date: 9/2/08

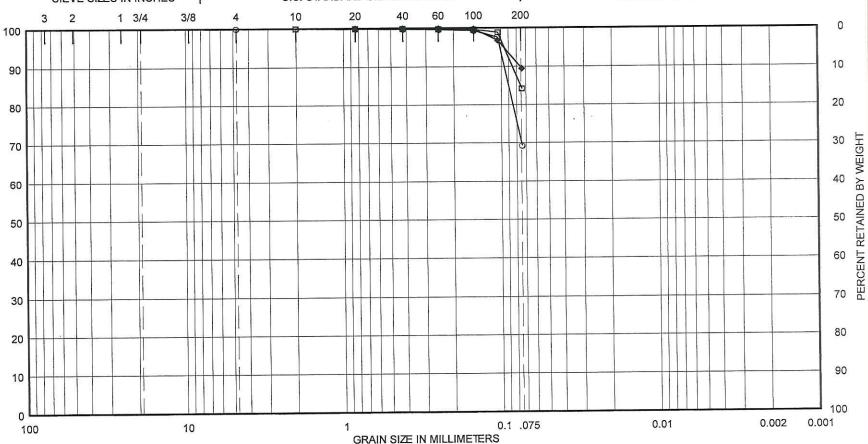
Drawn by: Tomol

Date: 9/2/08









GRAV	'EL		SAND		SILT or CLAY
Coarse	Fine	Coarse	Medium	Fine	, SILT OF CLAT

SAMPLE NO.	DEPTH, FT	SYMBOL	CLASSIFICATION
14	17.00	0	SANDY SILT (ML) with a few clay pockets and shell fragments
17	24.50		SILT (ML) with sand and many clay pockets
23	44.50	•	SILT (ML) with a trace of sand, an H2S odor and a few clay seams

# **GRAIN-SIZE DISTRIBUTION CURVES**



Checked by: KB

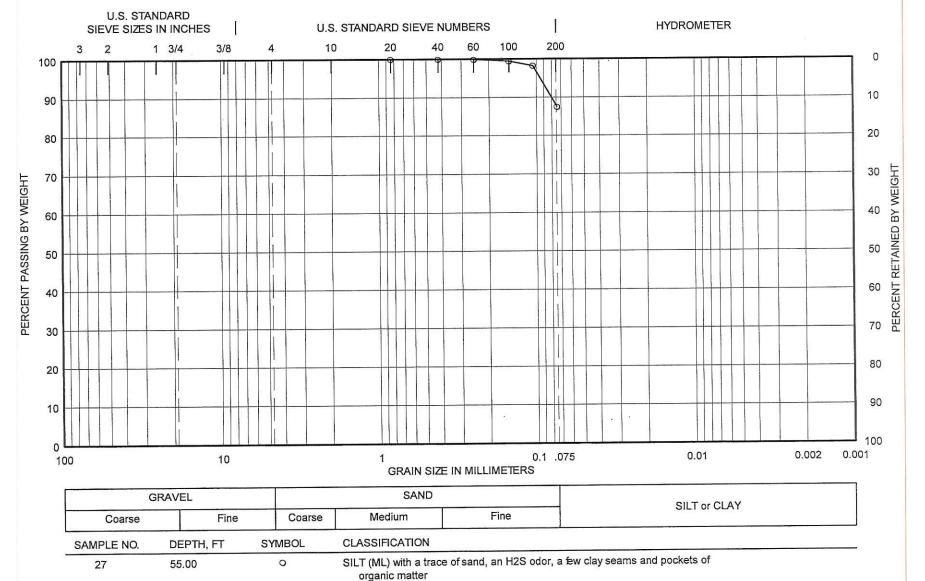
Date: 7/2/08

Approved by: 09

Date: 9/2/08

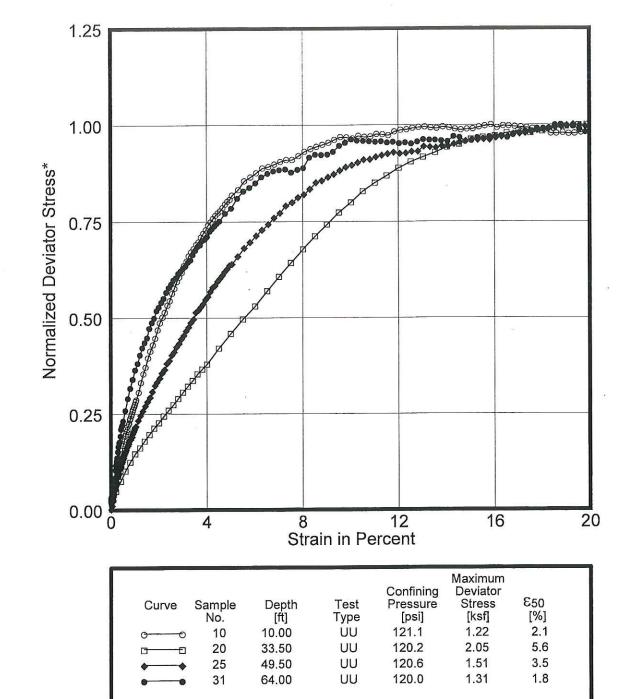
Drawn by: Tlomol

Date: 9/2/08



# **GRAIN-SIZE DISTRIBUTION CURVES**





# STRESS-STRAIN CURVES Unconsolidated-Undrained Triaxial Compression Test

Texas Offshore Port System, SPM #2 Anchor Leg #6 Block A-36, Galveston Area

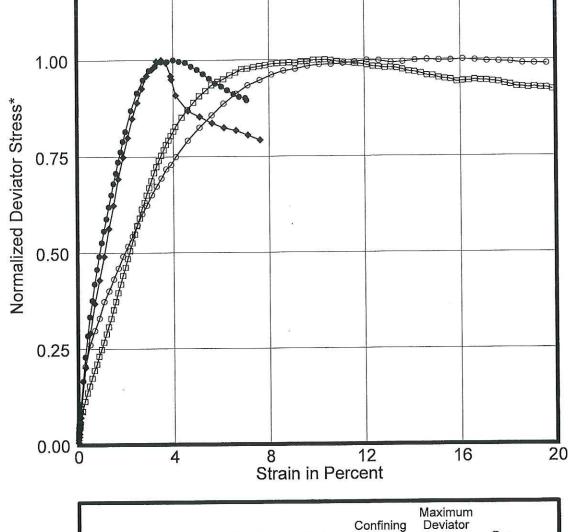
\* Normalized with respect to maximum deviator stress.

Report No. 0201-6501

Julecked By:

PLATE A-16a

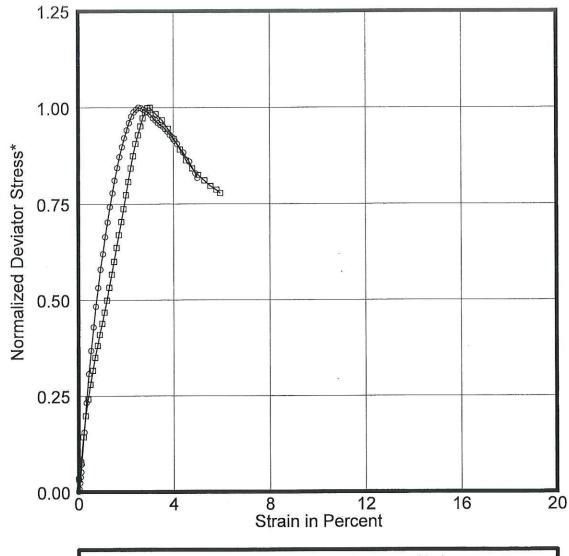
1.25



				Confining	Maximum Deviator	
Curve	Sample No.	Depth [ft]	Test Type	Pressure [psi]	Stress [ksf]	£50 [%]
0-0	35	67.50	ŬÜ	120.3	1.45	2.0
<del></del>	37	74.50	UU	119.9	1.75	2.2
•	40	85.50	UU	120.2	3.32	1.1
	43	95.50	UU	121.2	2.94	0.9

<sup>\*</sup> Normalized with respect to maximum deviator stress.

# STRESS-STRAIN CURVES Unconsolidated-Undrained Triaxial Compression Test



Curve	Sample No.	Depth [ft]	Test Type	Confining Pressure [psi]	Maximum Deviator Stress [ksf]	850 [%]
00	49	115.50	ŬŪ	120.5	3.28	0.8
<del>a - a</del>	54	130.50	UU	119.1	4.18	1.2

<sup>\*</sup> Normalized with respect to maximum deviator stress.

# STRESS-STRAIN CURVES

Unconsolidated-Undrained Triaxial Compression Test

## C) STARFIX.NAV®

**STARFIX.NAV**® is an on board computer graphic system interfaced to the primary positioning system capable of displaying real time position of a vessel in relation to known hazards, fairways, proposed location, etc. **DRONE**® units when used on anchor handling vessels utilize Differential GPS transmitted to the master station via radio telemetry link to display in real time the position of that vessel.

D) Vessel orientation by Sperry SR 50 Mod 1 North Seeking Gyro or a S. G. Brown Meridian North Seeking Gyro

North seeking Gyro compass. Accuracy ± 2° after 4 hours initial spin up.

E) Scanning Sonar

Simrad MS1000 High Resolution Sonar

### 5. RESULTS:

Geographic positions are based on Clarke 1866 Spheroid, North American Datum 1927. Grid coordinates are based on Texas South Central Zone Lambert, NAD 27.

Field operations were conducted from June 26, 2008 to June 28, 2008 with the following results:

A) STARFIX® position derived by averaging readings over a one hour period at an update rate of 750 ms. per reading.

# CORE 1(Leg #2 - West)

Y = 264,853.21 X = 3,275,180.44

Latitude: 28° 30' 07.937" N Longitude: 95° 01' 44.907" W

This location being 3493.21' FSL and 5304.63' FWL of Block A36, Galveston Area

### CORE 2 (Leg #6 - East)

Y = 270,117.00' X = 3,284,733.18'

Latitude: 28° 30' 56.803" N Longitude: 94° 59' 55.883" W

This location being 7083.00' FNL and 982.63' FEL of Block A36, Galveston Area

## **CORE 3 (PLET #1)**

Y = 265,270.45' X = 3,276,615.21'

Latitude: 28° 30' 11.583" N Longitude: 95° 01' 28.676" W

This location being 3910.45' FSL and 6739.40' FWL of Block A36, Galveston Area

# **CORE 4 (PLET #2)**

Y = 269,117.95' X = 3,283,617.17'

Latitude: 28° 30' 47.295" N Longitude: 95° 00' 08.769" W

This location being 7757.95' FSL and 2098.64' FEL of Block A36, Galveston Area

# CORE 4A (PLET #2 - Amended)

Y = 269,121.01' X = 3,283,587.30'

Latitude: 28° 30' 47.335" N Longitude: 95° 00' 09.102" W

This location being 7761.01' FSL and 2128.51' FEL of Block A36, Galveston Area

### 6. CONFIRMATION:

DGPS was used for confirmation.

The results were as follows:

CORE 1 (Leg <u>#2 - West)</u>	CORE 2 (Leg #6 - <u>East)</u>	CORE 3 (PLET #1)
Y = 264,853' X = 3,275,181'	Y = 270,117' X = 3,284,733'	Y = 265,271' X = 3,276,615'
CORE 4 (PLET #2)	CORE 4A (PLET #2 - Amend	ded)

$$Y = 269,118'$$
  $Y = 269,121'$   
 $X = 3,283,617'$   $X = 3,283,588'$ 

### 7. HSE INCIDENTS:

No incidents.



	Tin	<u>ne</u>	
<u>Date</u>	<u>From</u>	<u>To</u>	Description of Activities
June 28, 2008	***	1245	Arrive in Block A-36, Galveston Area, SPM #2 Anchor Leg #6 location onboard the <i>R/V Seaprobe</i> .
	1245	1330	Set 4-point anchor spread.
	1330	1350	Rig up to drill and sample.
	***	1350	Estimate water depth of 106 ft using echo sounder and 109 ft using wireline technique.
	1350	1405	Perform scanning sonar survey.
	***	1400	Conduct task change meeting.
	1405	1450	Run drill pipe to mudline.
	. ****	1450	Measure water depth of 110 ft using pipe tally/bottom sensor and 109.5 ft using pressure transducer.
	1450	2020	Drill and sample. Boring terminated at 131-ft penetration.
	2020	2105	Pull drill pipe above mudline and reposition vessel.
	****	2105	Measure supplemental water depth of 111 ft using bottom sensor/pipe tally and 109.2 ft using pressure transducer.
	2105	2140	Pull drill pipe to deck and secure equipment for travel.
	2140	2230	Pull anchors.
	2230	***	Depart location.

# **SUMMARY OF FIELD OPERATIONS**



## C) STARFIX.NAV®

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CORE 4 (PLET #2)	CORE 4A (PLET #2 - Amend	ded)

$$Y = 269,118'$$
  $Y = 269,121'$   
 $X = 3,283,617'$   $X = 3,283,588'$ 

### 7. HSE INCIDENTS:

No incidents.

FINAL SOIL BORINGS								
LOCATION	CALLNS	CALLEW	X COORDINATE	Y COORDINATE	LATITUDE	LONGITUDE		
CORE 1	3,493,21' FSL	5,304.63' FWL	3,275,180.44	264,853.21'	28' 30' 07.937"N			
CORE 2	7,083.00' FNL	982.63' FEL	3,284,733.18	270,117.00'	28' 30' 56.803"N	94° 59' 55.883"W		
CORE 3	3,910.45' FSL	6,739,40' FWL	3,276,615.21	265,270.45'	28' 30' 11.583"N	95' 01' 28.676"W		
CORE 4	7,757.95' FSL	2,098.64' FEL	3,283,617.17	269,117.95'	28' 30' 47.295"N	95' 00' 08.769"W		
CORE 4A	7.761.01' FSL	2,128.51' FEL	3,283,587.30'	269,121.01	28' 30' 47.335"N	95' 00' 09.102"W		

O CORE 2

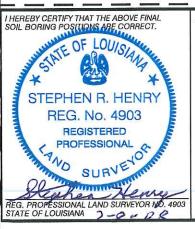
GAA36

CORE 4A CORE 4

O CORE 3

O CORE 1

GRID



1) SURVEYED COORDINATES TRANSFORMED FROM NADBS (GPS DATUM) TO NAD27 (CHART DATUM) USING NADCON VERSION 2.1.

# **ENTERPRISE FIELD SERVICES, LLC**

**FINAL SOIL BORINGS** (PROP. ANC & PLET) **NO LEASE NUMBER** 

> **BLOCK A36 GALVESTON AREA GULF OF MEXICO**

FUGRO CHANCE INC. 200 Dulles Dr. Lafqyelle, Leuisione 70506-3001 (337) 237-1300

fueno

2,000

Of:

GEODETIC DATUM: NAD27 SCALE PROJECTION: TEXAS SOUTH CENTRAL GRID UNITS: US SURVEY FEET IN FEET

Job No.: 08-01930 | Date: 7/8/08 Chart: Drwn: TCG

Printed: 7/8/08 Dwgfile: O:\WellPermit\TXsc\GA\Permit\A36\_CORE\_NoLease\_0801930