# EUSTIS ENGINEERING SINCE 1946

# LOG OF BORING AND TEST RESULTS

# Ducks Unlimited, Inc. Pierce Marsh Beneficial Use Marsh Creation Phase 1 North of West Bay Near Galveston Island

Galveston County, Texas

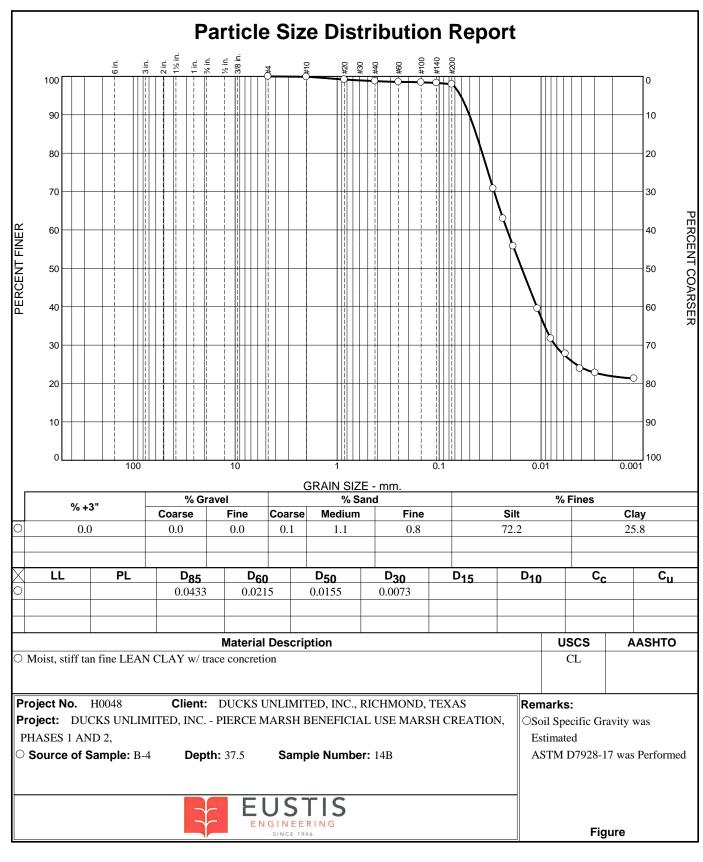
Boring: B-4

Project No: H0048 Date: 07/16/2022 Latitude: 29.31479° Longitude: -94.95743°

Water Depth: See Text Total Depth: 40.0 ft

### Density **Shear Tests** Atterberg Limits Scale in Water Depth PΡ SPT Sample Visual Classification L Symbol USC Content Other Tests Feet Number in Feet Drv Wet ф LL PLы R % Type pcf pcf psf 0 Moist, soft gray FAT CLAY w/few organic matter & roots 39 0.50 95 121 ОВ 0 185 1B ĭ 27 w/trace of fine sand pockets & roots 2A 2 27 27 1.00 58 18 40 2B Moist, medium stiff to stiff gray & tan FAT CLAY w/trace of fine sand pockets ЗА 26 21 23 11 4 5 6 7 5 1.00 3B 106 129 OB 0 946 4A 1.00 4B 8 29 30 5A w/few fine sand pockets 92 1.00 5B 9 120 OB 0 810 10 6A 10 31 33 6B 11 1.00 Moist, stiff reddish-tan, reddish-brown, & gray FAT CLAY w/trace of fine sand pockets 33 35 7A 12 СН 1.00 7B 13 87 117 OB 0 1144 30 29 14 8A EUSTIS\_GINT\_LIBRARY\_4-18-2022.GLB EE STANDARD BORING LOG H0048.GPJ 8/18/22 15 15 1.00 8B 16 30 9A 31 1.00 9B 17 72 21 51 CON 85 10A 18 1.00 19 27 58 17 41 20 w/few fine sand pockets 11A 23 32 90 119 1.00 NS 24 25 w/trace of fine sand pockets 28 12A 31 1.00 12B 29 28 30 33 13A 37 0.50 13B 34 29 92 119 35 CL Moist, stiff tan fine LEAN CLAY 14A 38 26 0.50 14B 39 27 PD 40 45

NOTES: Boring 4 was drilled in 3 in. of water.



Tested By: BH & KP Checked By: CD

## **GRAIN SIZE DISTRIBUTION TEST DATA**

8/11/2022

Client: DUCKS UNLIMITED, INC., RICHMOND, TEXAS

Project: DUCKS UNLIMITED, INC. - PIERCE MARSH BENEFICIAL USE MARSH CREATION, PHASES 1 AND

2,

NORTH OF WEST BAY NEAR GALVESTON ISLAND, GALVESTON COUNTY, TEXAS. DU CONTRACT NO. TX-0-2. DU PROJECT NO. TX-194-4. DU TASK ORDER NO. 1

**Project Number:** H0048

Location: B-4
Depth: 37.5

Depth: 37.5 Sample Number: 14B

Material Description: Moist, stiff tan fine LEAN CLAY w/ trace concretion

**USCS Classification:** CL

**Testing Remarks:** Soil Specific Gravity was Estimated ASTM D7928-17 was Performed

Tested by: BH & KP Checked by: CD

# **Sieve Test Data**

Post #200 Wash Test Weights (grams): Dry Sample and Tare = 1.61

Tare Wt. = 0.00

Minus #200 from wash = 97.9%

Dry Sample and Tare (grams)	Tare (grams)	Cumulative Pan Tare Weight (grams)	Sieve Opening Size	Cumulative Weight Retained (grams)	Percent Finer	Percent Retained
75.86	0.00	0.00	#4	0.00	100.0	0.0
			#10	0.08	99.9	0.1
			#20	0.62	99.2	0.8
			#40	0.93	98.8	1.2
			#60	1.09	98.6	1.4
			#100	1.20	98.4	1.6
			#140	1.30	98.3	1.7
			#200	1.55	98.0	2.0

# **Hydrometer Test Data**

Hydrometer test uses material passing #10

Percent passing #10 based upon complete sample = 99.9

Weight of hydrometer sample =75.86

Automatic temperature correction

Composite correction (fluid density and meniscus height) at 20 deg. C = -6.06

Meniscus correction only = 0.9Specific gravity of solids = 2.70

Hydrometer type = 152H

Hydrometer effective depth equation: L = 16.294964 - 0.164 x Rm

Elapsed Time (min.)	Temp. (deg. C.)	Actual Reading	Corrected Reading	K	Rm	Eff. Depth	Diameter (mm.)	Percent Finer	Percent Retained
1.25	22.0	60.0	54.3	0.0131	60.9	6.3	0.0295	70.8	29.2
2.25	22.0	54.0	48.3	0.0131	54.9	7.3	0.0236	63.0	37.0
4.00	22.0	48.5	42.8	0.0131	49.4	8.2	0.0188	55.8	44.2
15.00	22.0	36.0	30.3	0.0131	36.9	10.2	0.0108	39.5	60.5
30.00	22.0	30.0	24.3	0.0131	30.9	11.2	0.0080	31.7	68.3
60.00	22.0	27.0	21.3	0.0131	27.9	11.7	0.0058	27.8	72.2
120.00	22.0	24.0	18.3	0.0131	24.9	12.2	0.0042	23.9	76.1

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Hydrometer Test Data (continued)										
Elapsed Time (min.)	Temp. (deg. C.)	Actual Reading	Corrected Reading	K	Rm	Eff. Depth	Diameter (mm.)	Percent Finer	Percent Retained	
241.50	22.6	23.0	17.5	0.0130	23.9	12.4	0.0029	22.8	77.2	
1440.00	22.0	22.0	16.3	0.0131	22.9	12.5	0.0012	21.3	78.7	

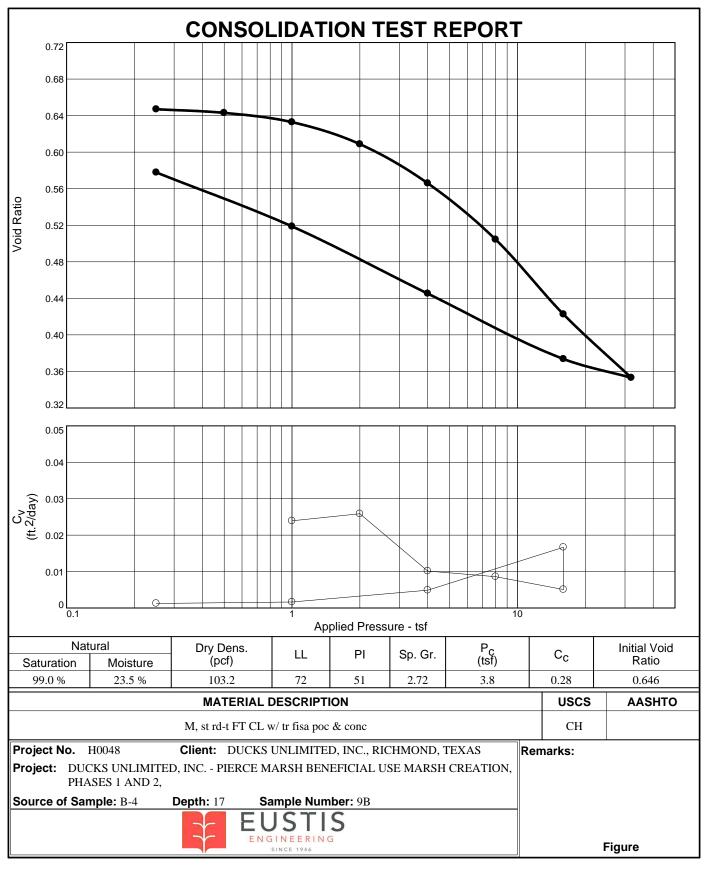
# Fractional Components

Cobbles	Gravel			Sand				Fines		
Copples	Coarse	Fine	Total	Coarse	Medium	Fine	Total	Silt	Clay	Total
0.0	0.0	0.0	0.0	0.1	1.1	0.8	2.0	72.2	25.8	98.0

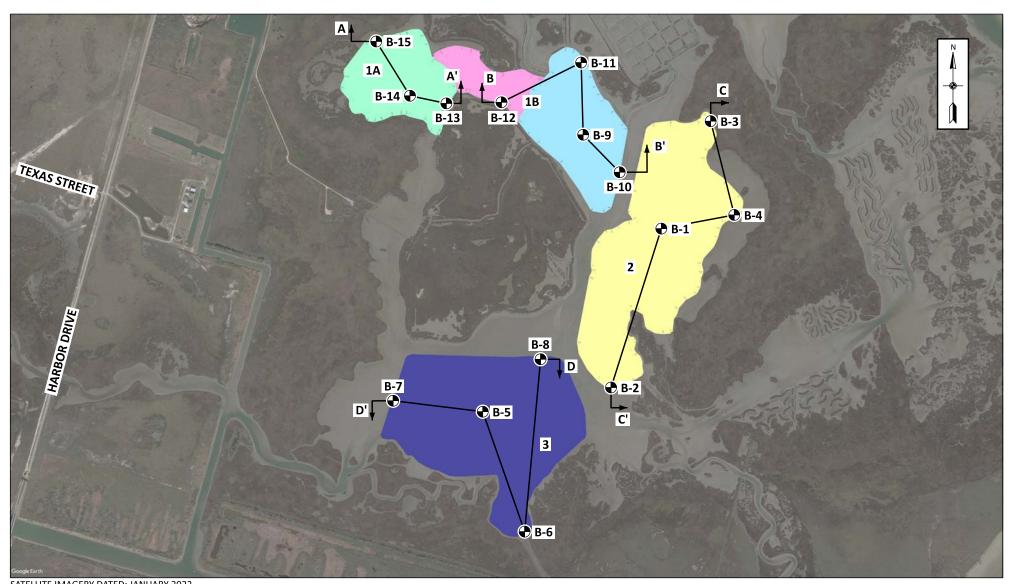
D <sub>5</sub>	D <sub>10</sub>	D <sub>15</sub>	D <sub>20</sub>	D <sub>30</sub>	D <sub>40</sub>	D <sub>50</sub>	D <sub>60</sub>	D <sub>80</sub>	D <sub>85</sub>	D <sub>90</sub>	D <sub>95</sub>
				0.0073	0.0110	0.0155	0.0215	0.0377	0.0433	0.0506	0.0615

Fineness Modulus 0.05

\_\_\_\_\_ Eustis Engineering L.L.C.



Tested By: BH Checked By: RR



SATELLITE IMAGERY DATED: JANUARY 2022

NOT TO SCALE

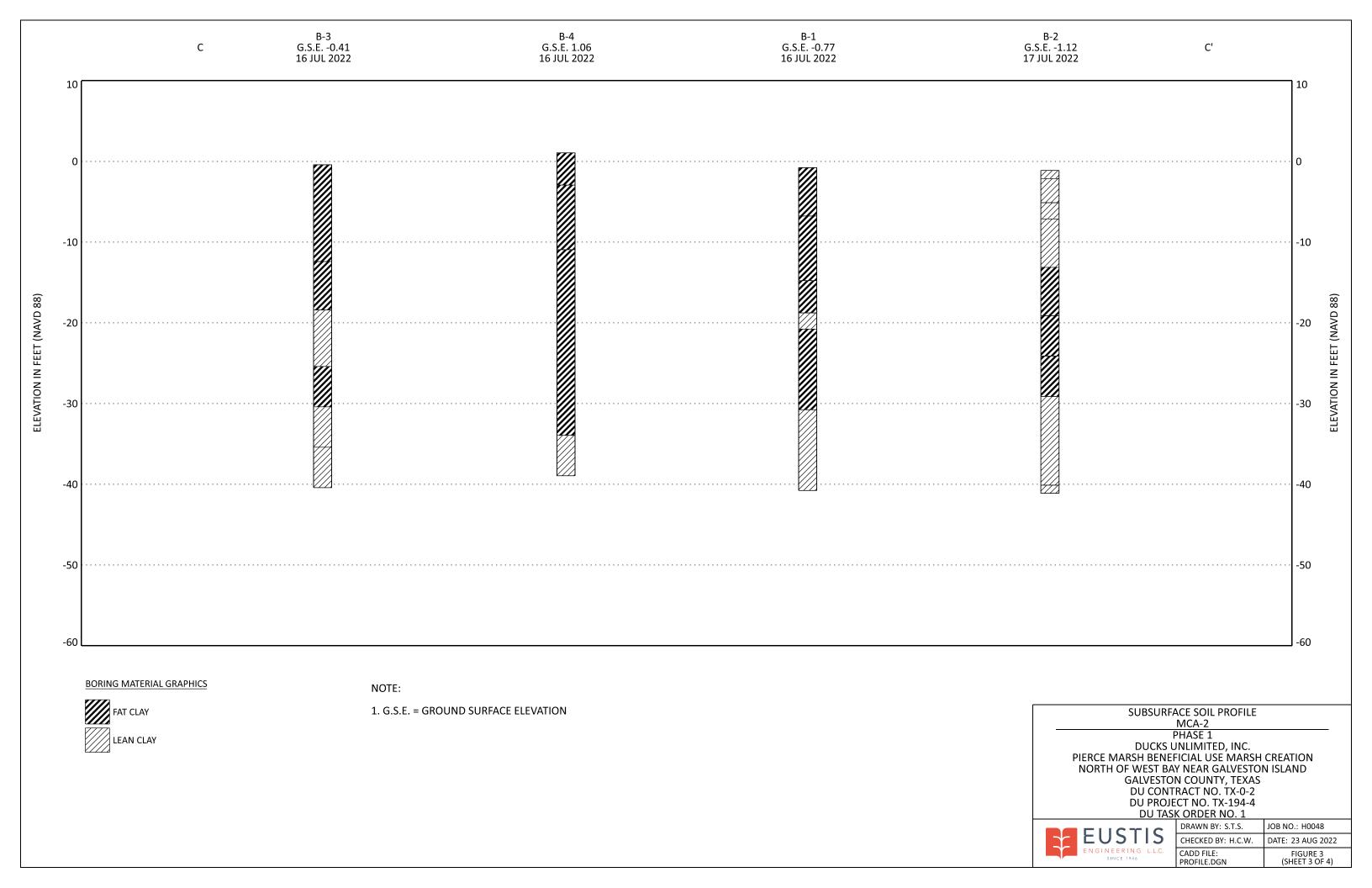
DENOTES APPROXIMATE LOCATIONS OF SOIL BORINGS DRILLED BETWEEN 11 AND 18 JULY 2022

## **BORING LOCATION PLAN**

PHASE 1
DUCKS UNLIMITED, INC.
PIERCE MARSH BENEFICIAL USE MARSH CREATION
NORTH OF WEST BAY NEAR GALVESTON ISLAND
GALVESTON COUNTY, TEXAS
DU CONTRACT NO. TX-0-2
DU PROJECT NO. TX-194-4
DU TASK ORDER NO. 1



R ORDER NO. 1							
DRAWN BY: S.T.S.	JOB NO.: H0048						
CHECKED BY: H.C.W.	DATE: 15 AUG 2022						
CADD FILE: LOCATION PLAN.DGN	FIGURE 2						





# LEGEND AND NOTES FOR LOG OF BORING AND TEST RESULTS

PP Pocket penetrometer: Resistance in tons per square foot Standard Penetration Test: Number of blows of a 140-lb hammer dropped 30 inches required to SPT drive 2-in. O.D., 1.4-in. I.D. sampler a distance of 1 foot into the soil after first seating it 6 inches. Values shown have not been corrected. Shelby SPT Auger Uvibracore Type of Sampling **SPLR** SYMBOL Clay Silt Peat/Humus Shells Stone/Gravel Sand Predominant type shown heavy; modifying type shown light USC **Unified Soil Classification** 

SHEAR TESTS
TYPE

UC Unconfined compression shear

DENSITY Unit weight in pounds per cubic foot

OB Unconsolidated undrained triaxial compression shear on one specimen confined at the approximate overburden pressure

UU Unconsolidated undrained triaxial compression shear

Angle of internal friction in degrees

c Cohesion in pounds per square foot

# ATTERBERG LIMITS

LL Liquid Limit

PL Plastic Limit

PI Plasticity Index

## **OTHER TESTS**

CON Consolidation

-#200 Percent passing a U.S. No. 200 sieve

SV Particle size distribution (sieve only)

PD Particle size distribution (sieve and hydrometer)

k Coefficient of permeability in centimeters per second

SP Swelling pressure in pounds per square foot

Other laboratory test results reported on separate figures

# **GENERAL NOTES**

- (1) If a ground water depth is shown on the boring log, these observations were made at the time of drilling and were measured below the existing ground surface. These observations are shown on the boring logs. However, ground water levels may vary due to seasonal fluctuations and other factors. If important to construction, the depth to ground water should be determined by those persons responsible for construction immediately prior to beginning work.
- (2) While the individual logs of borings are considered to be representative of subsurface conditions at their respective locations on the dates shown, it is not warranted that they are representative of subsurface conditions at other locations and times.