

U.S. ARMY CORPS OF ENGINEERS

BKM

DEPTH, FEET	SAMPLE NO.	PEN./TORVANE SPT.-BLOW COUNT	BORING NO. <u>91-47</u> DATE: BEGIN <u>03-10-91</u> PAGE <u>1 1 /</u> JOB NO. <u>149446</u> COMPLETE <u>03-10-91</u> Thin Walled Tube PROJECT <u>CHANNEL TO VECTORIA</u> <input checked="" type="checkbox"/> 3" <input type="checkbox"/> 6" LOCATION <u>VECTORIA CHANNEL</u> ELEVATION OF HOLE _____ MANUFACTURER'S DESIGNATION OF DRILL RIG <u>F-36</u> GROUNDWATER: DEPTH <u>4</u> ft., ELEV. _____ ft., at end of Drilling WEATHER <u>SUNNY &amp; CLEAR</u> DRILLER <u>ROBERT TANKE</u> LOGGER <u>TOPPA GENTR</u>				
			COLOR	MATERIAL TYPE	CONSISTENCY	SECONDARY CONSTITUENTS	STRUCTURAL FEATURES AND COMMENTS
0	1	1.75	GRAY	CLAY	STIFF		-w/ROOTS 0'-2'
	2	2.0	LIGHT GRAY*	"	VERY STIFF		-w/SAND POCKETS 2'-6' +TAN
5	3	2.25	"	"	"		
	4	2.25	"	"	"		
	5	2.25	"	"	"		
10	6	0.75	LIGHT GRAY*	CLAY	MEDIUM STIFF	SANDY	+TAN -w/SAND LAYERS +POCKET 10'-
	7	0.25	"	"	SOFT	"	
15	8	0.25	GRAY	"	MEDIUM		-w/SHELL FRAGMENTS 14'-18'
	9	0.5	"	"	STIFF		
	10	0.50	"	"	"		
20	11	0.25	"	"	SOFT		
	12	0.5	"	"	MEDIUM STIFF		-w/SHELL FRAGMENTS 22-
25	13	0.5	"	"	"		-w/SANDY CLAY LAYERS 24'-26'
	14	0.5	"	"	"		
	15	5/5/5	LIGHT GRAY	SAND	Medium Dense		
30							BT TO M' OF 9147
35							

Project : Channel enlargement channel to Victoria, Victoria, Texas

**SUMMARY OF LABORATORY TEST RESULTS**

Boring No.

91-47 EL. 5.8' NGVD

Contract No. DACW64-91-D-0001 Delivery Order No.0009

S #	Depth (ft)	P P (tsf)	SPT Blows per Foot	Visual Classification	USC	M c (%)	Dry Unit Wt (pcf)	Wet Unit Wt (pcf)	LL (%)	PL (%)	Mechanical Analysis % Passing					Torvane Shear Strength (tsf)	q u (tsf)
											#4	#10	#40	#100	#200		
											1	0-2	3.00		Dark gray&yellowish brown,Clay,Very stiff,Silty		
2	2-4	2.50		Yellowish brown&gray,Clay,Very stiff,Silty	CL	20.3	107.1	128.9	32	16	100.0	99.9	99.9		90.2		1.20
3	4-6	1.50		Yellowish brown&gray,Clay,Stiff,Silty	CL	20.6											
4	6-8	2.25		Gray,Clay,Stiff,Silty	CL	24.8											
5	8-10	1.75		Gray,Clay,Stiff,Silty	CL	30.1											
6	10-12	0.50		Gray&yellowish brown,Clay,Medium stiff,silty	CL	31.3	93.4	122.7	38	18	100.0	100.0	99.8		99.5	0.35 .70	38
7	12-14	0.50		Gray&yellowish brown,Clay,Medium stiffw/sand parting	CH	27.0										0.25 .50	
8	14-16	0.50		Gray,Clay,Medium stiff,w/shell fragments	CH	36.2										0.25 .50	
9	16-18	0.50		Gray,Clay,Medium stiff,w/shell fragments	CH	30.4										0.30 .60	
10	18-20	0.50		Gray,Clay,Medium stiff,w/shell fragments	CH	47.8										0.30 .60	
11	20-22	0.25		Dark gray,Clay,Soft,w/sand pockets	CH	48.4	75.0	111.3	55	24	100.0	100.0	99.9		99.3	0.30 .60	.58
12	22-24	0.25		Gray,Clay,Soft,w/sand pockets	CH	46.0										0.30 .60	
13	24-26	0.00		Dark gray,Clay,Very soft,w/shell fragments	CH	42.6	80.5	114.8	53	22	99.6	98.4	92.2*		64.6	0.10 .20	.12
14	26-28	0.00		Olive gray,Sand,Very loose,w/clay pockets	SC	27.9										0.05 .10	
15	28.5-30		10	Gray,Sand,Medium dense,Silty	SM						99.8	99.6	91.0		15.8	0.00 0.00	

S # : Sample Number, P P : Pocket Penetrometer Reading, U S C : Unified Soil Classification, M c : Moisture Content

q u : Unconfined Compressive Strength, W O H : Weight of hammer, W O P : Weight of pipe

\*Material coarser than No.40 sieve are shell fragments

JOB NO. 14G459

DATE 4/28/91

PROJECT CHANNEL ENLARGEMENT, CHANNEL TO VICTORIA, TEXAS

BOILING NO. 91-47

SAMPLE NO. 2

DEPTH 2-4 ft

SPECIMEN NO. 1

CLASSIFICATION

Yellowish brown & gray, clay, very stiff, silty,

Tare No.	P-13	Height	5.595 in.
Tare plus Wet Specimen	728.05 gm	Average Diameter	2.830 in.
Tare plus Dry Specimen	612.28 gm	Initial Area	6.290 sq in.
Water Weight	115.77 gm	Volume	35.194 cu in.
Tare Weight	42.30 gm	Volume of Solids	cu in.
Wet Specimen	1190.77 gm	Void Ratio	
Dry Specimen	989.74 gm	Saturation	%
Water Content	20.31 %	Dry Density	107.1 lb/cu ft
Specific Gravity of Solids			
LL = 32	PL = 16	PI = 16	

Proving Ring No. 10170

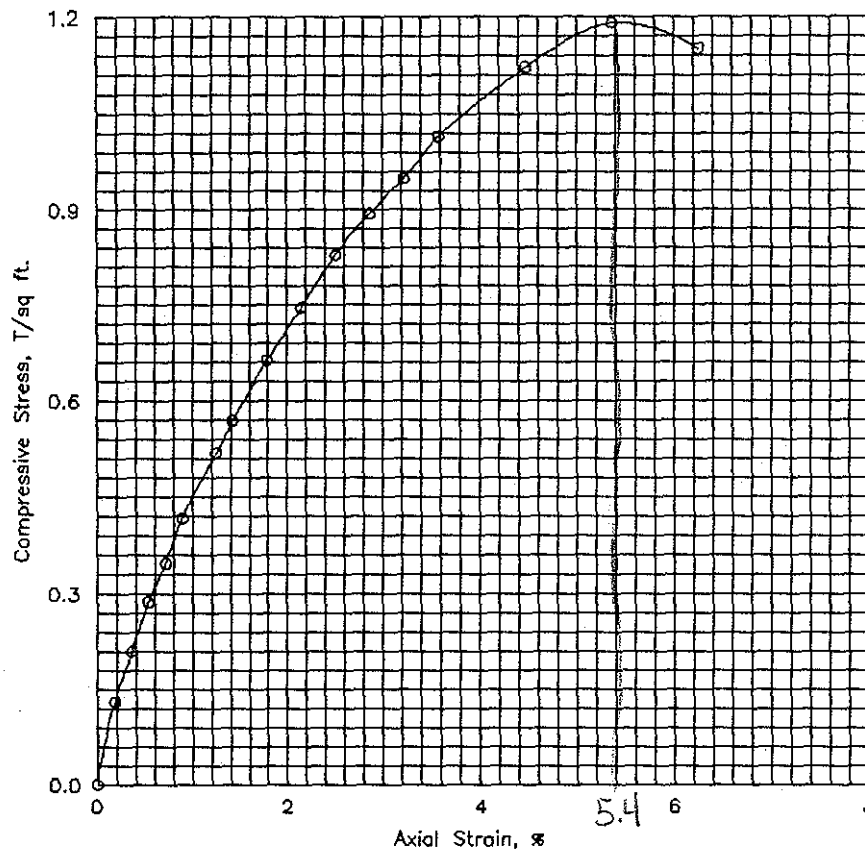
Proving Ring Constant, K = .766 lbs/div.

Elapsed Time min.	Dial Reading 0.001"	Cumulative Change in.	Proving Ring Dial Reading	Axial Load lb	Axial Strain	Area Corr. sq in.	Compr. Stress tsf
.0	0.	.000	.0	.0	.000	6.29	.000
.2	10.	.010	15.0	11.5	.002	6.30	.131
.4	20.	.020	24.0	18.4	.004	6.31	.210
.6	30.	.030	33.0	25.3	.005	6.32	.288
.8	40.	.040	40.0	30.6	.007	6.34	.348
.9	50.	.050	48.0	36.8	.009	6.35	.417
1.2	70.	.070	60.0	46.0	.013	6.37	.519
1.5	80.	.080	66.0	50.6	.014	6.38	.570
1.8	100.	.100	77.0	59.0	.018	6.40	.663
2.1	120.	.120	87.0	66.6	.021	6.43	.746
2.5	140.	.140	97.0	74.3	.025	6.45	.829
2.8	160.	.160	105.0	80.4	.029	6.48	.894
3.2	180.	.180	112.0	85.8	.032	6.50	.950
3.5	200.	.200	120.0	91.9	.036	6.52	1.015
4.2	250.	.250	134.0	102.6	.045	6.58	1.122
5.8	300.	.300	143.5	109.9	.054	6.65	1.191
6.2	350.	.350	140.0	107.2	.063	6.71	1.151

Job No. 14G459

EM 1110-2-1906  
Appendix XI  
30 Nov 70

Failure Sketches



- Controlled stress
- Controlled strain

Test No.		1	
Type of Specimen		Undisturbed	
Initial	Water content	$w_D$	20.3 %
	Void ratio	$e_0$	
	Saturation	$S_0$	%
	Dry density, lb/cu ft	$\gamma_d$	107.1
Time to failure, min		$t_f$	5.77
Unconfined compressive strength, T/sq ft		$q_u$	1.19
Undrained shear strength, T/sq ft		$S_u$	.60
Sensitivity ratio		$S_t$	
Initial specimen diameter, in.		$D_0$	2.830
Initial specimen height, in.		$H_0$	5.595
Classification Yellowish brown & gray, clay, very stiff, silty,			
LL	32	PL	16
		PI	16
		$G_s$	
Remarks		Project CHANNEL ENLARGEMENT, CHANNEL TO VICTORIA	
		Area Victoria, Texas	
		Boring No. 91-47	Sample No. 2
		Depth 2-4 ft	Date 4/28/91
		UNCONFINED COMPRESSION TEST REPORT	

JOB NO. 14G459

DATE 4/30/91

PROJECT CHANNEL ENLARGEMENT, CHANNEL TO VICTORIA, TEXAS

BORING NO. 91-47

SAMPLE NO. 6

DEPTH 10-12 ft

SPECIMEN NO. 1

CLASSIFICATION

Gray & yellowish brown, Clay, Medium stiff, Silty

Tare No.	P-21	Height	5.595 in.
Tare plus Wet Specimen	688.26 gm	Average Diameter	2.830 in.
Tare plus Dry Specimen	534.20 gm	Initial Area	6.290 sq in.
Water Weight	154.06 gm	Volume	35.194 cu in.
Tare Weight	42.58 gm	Volume of Solids	cu in.
Wet Specimen	1133.37 gm	Void Ratio	
Dry Specimen	862.95 gm	Saturation	%
Water Content	31.34 %	Dry Density	93.4 lb/cu ft
Specific Gravity of Solids			
LL = 38	PL = 18	PI = 20	

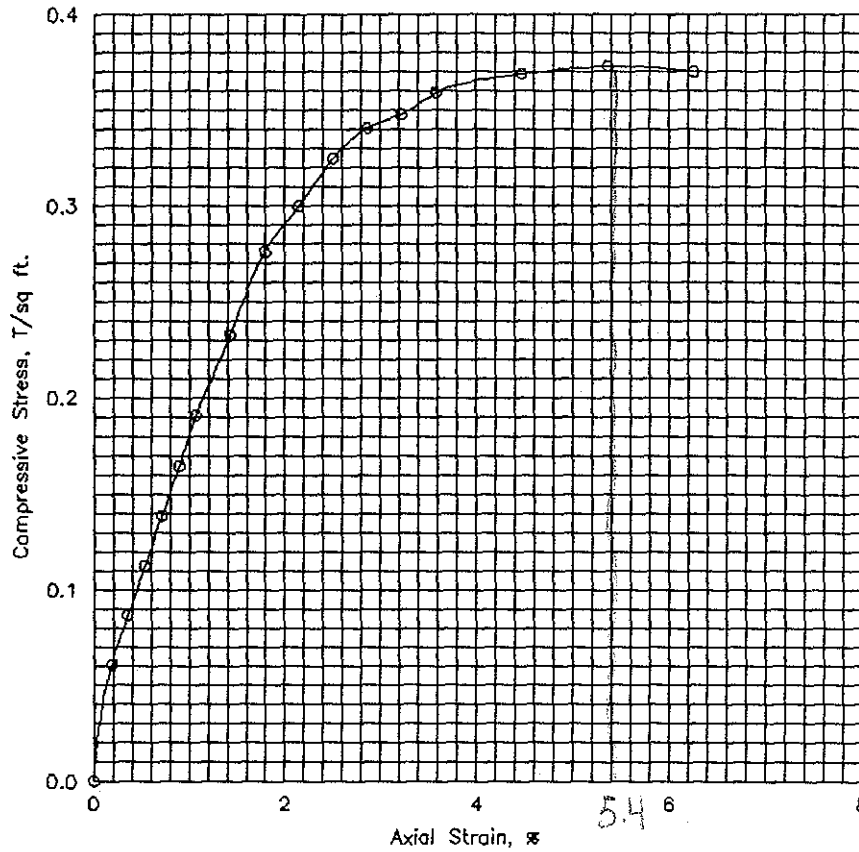
Proving Ring No. 10170  
 Proving Ring Constant, K = .766 lbs/div.

Elapsed Time min.	Dial Reading 0.001"	Cumulative Change in.	Proving Ring Dial Reading	Axial Load lb	Axial Strain	Area Corr. sq in.	Compr. Stress tsf
.0	0.	.000	.0	.0	.000	6.29	.000
.2	10.	.010	7.0	5.4	.002	6.30	.061
.4	20.	.020	10.0	7.7	.004	6.31	.087
.6	30.	.030	13.0	10.0	.005	6.32	.113
.7	40.	.040	16.0	12.3	.007	6.34	.139
.9	50.	.050	19.0	14.6	.009	6.35	.165
1.1	60.	.060	22.0	16.9	.011	6.36	.191
1.4	80.	.080	27.0	20.7	.014	6.38	.233
1.7	100.	.100	32.0	24.5	.018	6.40	.276
2.0	120.	.120	35.0	26.8	.021	6.43	.300
2.4	140.	.140	38.0	29.1	.025	6.45	.325
2.7	160.	.160	40.0	30.6	.029	6.48	.341
3.0	180.	.180	41.0	31.4	.032	6.50	.348
3.4	200.	.200	42.5	32.6	.036	6.52	.359
4.1	250.	.250	44.0	33.7	.045	6.58	.369
4.9	300.	.300	45.0	34.5	.054	6.65	.373
5.8	350.	.350	45.0	34.5	.063	6.71	.370

Job No. 14G459

EM 1110-2-1906  
Appendix XI  
30 Nov 70

Failure Sketches



- Controlled stress
- Controlled strain

Test No.		1			
Type of Specimen		Undisturbed			
Initial	Water content	$w_0$	31.3 %	%	%
	Void ratio	$e_0$			
	Saturation	$S_0$	%	%	%
	Dry density, lb/cu ft	$\gamma_d$	93.4		
Time to failure, min		$t_f$	4.93		
Unconfined compressive strength, T/sq ft		$q_u$	.37		
Undrained shear strength, T/sq ft		$S_u$	.19		
Sensitivity ratio		$S_t$			
Initial specimen diameter, in.		$D_0$	2.830		
Initial specimen height, in.		$H_0$	5.595		

Classification Gray & yellowish brown, Clay, Medium stiff, Silty

LL 38      PL 18      PI 20       $G_s$

Remarks \_\_\_\_\_  
\_\_\_\_\_  
\_\_\_\_\_  
\_\_\_\_\_

Project CHANNEL ENLARGEMENT, CHANNEL TO VICTORIA  
Area Victoria, Texas  
Boring No. 91-47      Sample No. 6  
Depth 10-12 ft      Date 4/30/91

UNCONFINED COMPRESSION TEST REPORT

JOB NO. 14G459

DATE 4/30/91

PROJECT CHANNEL ENLARGEMENT, CHANNEL TO VICTORIA, TEXAS

BORING NO. 91-47

SAMPLE NO. 11

DEPTH 20-22 ft

SPECIMEN NO. 1

CLASSIFICATION

Dark gray, Clay, Soft, w/sand pockets

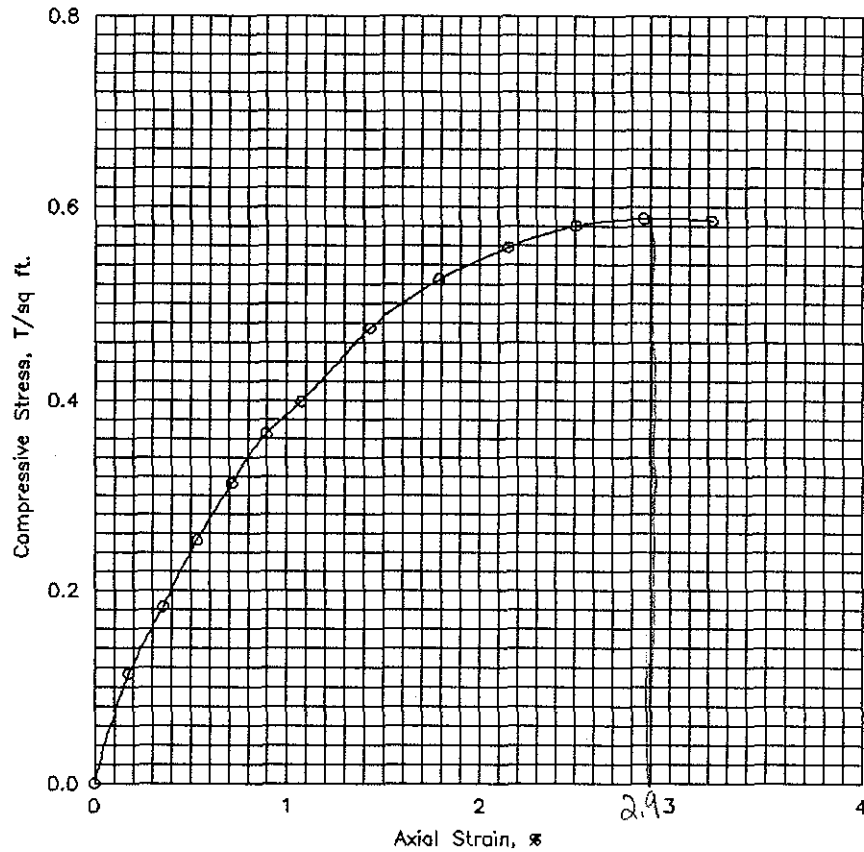
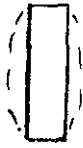
Tare No.	P-22	Height	5.595 in.
Tare plus Wet Specimen	557.41 gm	Average Diameter	2.830 in.
Tare plus Dry Specimen	389.62 gm	Initial Area	6.290 sq in.
Water Weight	167.79 gm	Volume	35.194 cu in.
Tare Weight	42.73 gm	Volume of Solids	cu in.
Wet Specimen	1028.22 gm	Void Ratio	
Dry Specimen	693.01 gm	Saturation	%
Water Content	48.37 %	Dry Density	75.0 lb/cu ft
Specific Gravity of Solids			
LL = 55	PL = 24	PI = 31	

Proving Ring No. 10170  
 Proving Ring Constant, K = .766 lbs/div.

Elapsed Time min.	Dial Reading 0.001"	Cumulative Change in.	Proving Ring Dial Reading	Axial Load lb	Axial Strain	Area Corr. sq in.	Compr. Stress tsf
.0	0.	.000	.0	.0	.000	6.29	.000
.2	10.	.010	13.0	10.0	.002	6.30	.114
.4	20.	.020	21.0	16.1	.004	6.31	.183
.6	30.	.030	29.0	22.2	.005	6.32	.253
.7	40.	.040	36.0	27.6	.007	6.34	.313
.9	50.	.050	42.0	32.2	.009	6.35	.365
1.1	60.	.060	46.0	35.2	.011	6.36	.399
1.4	80.	.080	55.0	42.1	.014	6.38	.475
1.7	100.	.100	61.0	46.7	.018	6.40	.525
2.0	120.	.120	65.0	49.8	.021	6.43	.558
2.4	140.	.140	68.0	52.1	.025	6.45	.581
2.7	160.	.160	69.0	52.9	.029	6.48	.588
3.0	180.	.180	69.0	52.9	.032	6.50	.586

Job No. 14G459

Failure Sketches



Controlled stress  
 Controlled strain

Test No.		1			
Type of Specimen		Undisturbed			
Initial	Water content	$w_0$	48.4 %	%	%
	Void ratio	$e_0$			
	Saturation	$S_0$	%	%	%
	Dry density, lb/cu ft	$\gamma_d$	75.0		
Time to failure, min		$t_f$	2.68		
Unconfined compressive strength, T/sq ft		$q_u$	.59		
Undrained shear strength, T/sq ft		$S_u$	.29		
Sensitivity ratio		$S_t$			
Initial specimen diameter, in.		$D_0$	2.830		
Initial specimen height, in.		$H_0$	5.595		

Classification Dark gray, Clay, Soft, w/sand pockets

LL	55	PL	24	PI	31	$G_s$
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Remarks	Project CHANNEL ENLARGEMENT, CHANNEL TO VICTORIA	
	Area Victoria, Texas	
	Boring No. 91-47	Sample No. 11
	Depth 20-22 ft	Date 4/30/91
	UNCONFINED COMPRESSION TEST REPORT	

JOB NO. 14G459

DATE 4/28/91

PROJECT CHANNEL ENLARGEMENT, CHANNEL TO VICTORIA, TEXAS

BOHNG NO. 91-47

SAMPLE NO. 13

DEPTH 24-26 ft

SPECIMEN NO. 1

CLASSIFICATION

Dark gray, Clay, Very soft, w/ shell fragments

Tare No.	P-20	Height	5.595 in.
Tare plus Wet Specimen	710.74 gm	Average Diameter	2.830 in.
Tare plus Dry Specimen	511.30 gm	Initial Area	6.290 sq in.
Water Weight	199.44 gm	Volume	35.194 cu in.
Tare Weight	42.67 gm	Volume of Solids	cu in.
Wet Specimen	1060.68 gm	Void Ratio	
Dry Specimen	744.03 gm	Saturation	%
Water Content	42.56 %	Dry Density	80.5 lb/cu ft
Specific Gravity of Solids			
LL = 53	PL = 22	PI = 31	

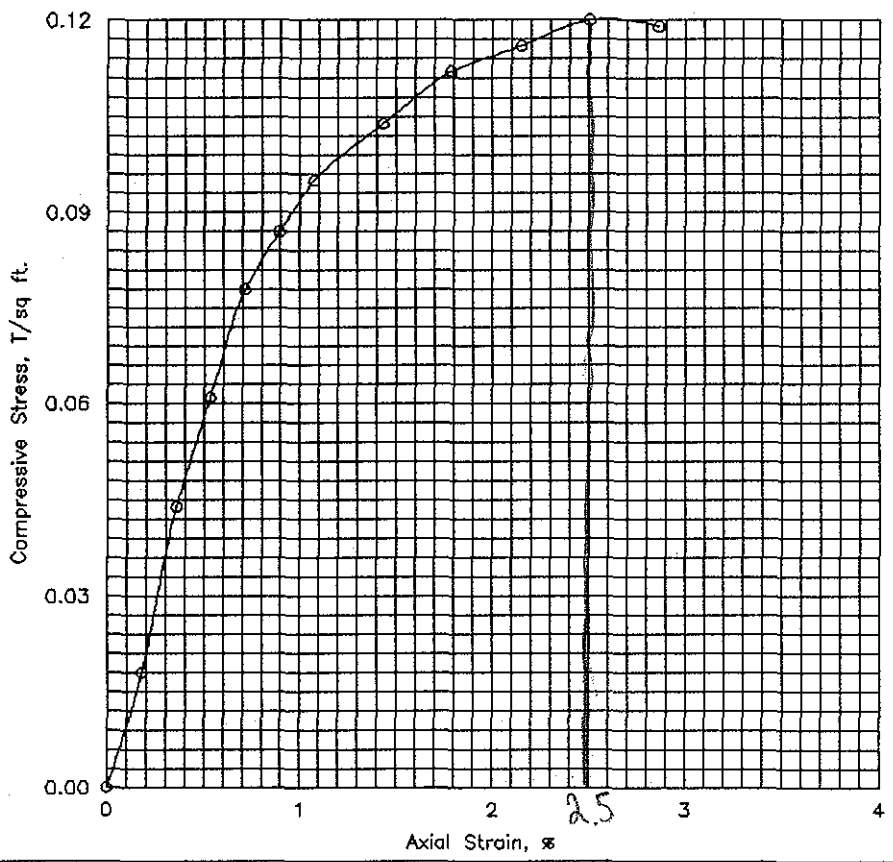
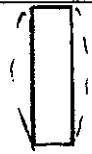
Proving Ring No. 10170

Proving Ring Constant, K = .766 lbs/div.

Elapsed Time n.	Dial Reading 0.001"	Cumulative Change in.	Proving Ring Dial Reading	Axial Load lb	Axial Strain	Area Corr. sq in.	Compr. Stress tsf
.0	0.	.000	.0	.0	.000	6.29	.000
.1	10.	.010	2.0	1.5	.002	6.30	.018
.3	20.	.020	5.0	3.8	.004	6.31	.044
.6	30.	.030	7.0	5.4	.005	6.32	.061
.9	40.	.040	9.0	6.9	.007	6.34	.078
1.8	50.	.050	10.0	7.7	.009	6.35	.087
1.2	60.	.060	11.0	8.4	.011	6.36	.095
1.5	80.	.080	12.0	9.2	.014	6.38	.104
1.8	100.	.100	13.0	10.0	.018	6.40	.112
2.2	120.	.120	13.5	10.3	.021	6.43	.116
2.5	140.	.140	14.0	10.7	.025	6.45	.120
2.8	160.	.160	14.0	10.7	.029	6.48	.119

Job No. 140459

Failure Sketches



Controlled stress  
 Controlled strain

Test No.		1			
Type of Specimen		Undisturbed			
Initial	Water content	$w_0$	42.6 %	%	%
	Void ratio	$e_0$			
	Saturation	$S_0$	%	%	%
	Dry density, lb/cu ft	$\gamma_d$	80.5		
Time to failure, min		$t_f$	2.50		
Unconfined compressive strength, T/sq ft		$q_u$	.12		
Undrained shear strength, T/sq ft		$S_u$	.06		
Sensitivity ratio		$S_t$			
Initial specimen diameter, in.		$D_0$	2.830		
Initial specimen height, in.		$H_0$	5.595		

Classification Dark gray, Clay, Very soft, w/ shell fragments

LL 53      PL 22      PI 31       $G_s$

Remarks	Project CHANNEL ENLARGEMENT, CHANNEL TO VICTORIA	
	Area Victoria, Texas	
	Boring No. 91-47	Sample No. 13
	Depth 24-26 ft	Date 4/28/91
	UNCONFINED COMPRESSION TEST REPORT	