

U.S. ARMY CORPS OF ENGINEERS

DEPTH, FEET		SAMPLE NO.	PEN. / TORVANE SPT. - BLOW COUNT	BORING NO. <u>91-285</u> DATE: BEGIN <u>12/30/94</u> PAGE <u>111</u>			JOB NO. <u>146-531</u> COMPLETE <u>12/30/94</u> Thin Walled Tube	
				PROJECT <u>G.I. WW</u>			☒ 3" ☐ 6"	
				LOCATION <u>Sanford Beach</u>			Sta <u>159+50</u>	
				ELEVATION OF HOLE <u>.</u>				
				MANUFACTURER'S DESIGNATION OF DRILL RIG <u>Falling CFD2</u>				
				GROUNDWATER: DEPTH <u>0</u> ft., ELEV. <u>.</u> ft., at end of Drilling				
				WEATHER <u>Cool Cloudy</u>				
				DRILLER <u>D. Gowan</u> LOGGER <u>D. Lowe II</u>				
				COLOR	MATERIAL TYPE	CONSISTENCY	SECONDARY CONSTITUENTS	STRUCTURAL FEATURES AND COMMENTS
0								
1	0.50			Tan	Clay	Medium stiff	silt	w/roots
2	1.0			Gray	Clay	stiff	silt	w/roots
5	1.25			Gray	Clay	soft	silt	w/roots
4	1.50			Gray	Clay	Medium stiff	silt	Tan
5	0.50			Gray	Clay	"	silt	Tan
10	0.50			Gray	Clay	"	silt	Tan
6	0.50			Gray	Clay	"	silt	Tan
7	0.50			Gray	Clay	"	silt	Tan
15	0.50			Gray	Clay	"	silt	Tan
9	2.0			Tan	clay	Very stiff	silt	Gray
20	0.50			Tan	clay	Medium stiff	silt	Gray
11	0.50			Tan	clay	"	silt	
12	0.50			Tan	clay	"	silt	Gray
25	0.75			Tan	clay	"	silt	Gray
								Bottom of 91-285 @ 26'
30								
35								

Project : Gulf Intracoastal Waterway, near Sargent Beach, Texas
Contract No. DACW64-92-D-0001 Delivery No. 0005

SUMMARY OF LABORATORY TEST RESULTS

Boring No. 91-285

S #	Depth (ft)	P P (tsf)	SPT Blows per Foot	Visual Classification	U S C	M c (%)	Dry Unit Wt (pcf)	Wet Unit Wt (pcf)	LL (%)	P L (%)	Mechanical Analysis % Passing					Torvane Shear Strength (tsf)	q u (tsf)
											#4	#10	#40	#100	#200		
1	0-2	0.50		Brown, Clay, Medium stiff, w/organic material and roots	CH	40.3											
2	2-4	1.25		Dark gray, Clay, Stiff, w/silt seams	CH	25.5			56								
3	4-6	0.75		Dark gray & yellowish gray, Clay, Medium stiff, w/silt pockets	CH	37.1										0.35	
4	6-8	0.50		Gray & yellowish brown, Clay, Medium stiff, w/ferrous stains, slickensided	CH	45.5	74.9	109.0								0.25	0.32
5	8-10	0.50		Gray & yellowish brown, Clay, Medium stiff, w/ferrous stains, slickensided	CH	42.2										0.25	
6	10-12	0.50		Brown, Clay, Medium stiff, Silty, w/blocky structure	CL	33.1	88.1	117.3	43	19	100.0	99.7	99.4	99.0	98.2	0.25	0.27
7	12-14	0.50 <i>0.25-0.75</i>		Gray & tan, Clay, Medium stiff, Silty <i>SWD</i>	<i>CL</i> CH	<i>33.8</i>	<i>85</i>		<i>55</i>	<i>18</i>					<i>91</i>		
8	14-16	1.00		Gray & brown, Clay, Stiff, w/silt pockets	CH	38.7										0.45	
9	16-18	1.50		Brown, Clay, Stiff, w/ferrous stains and blocky structure	CH	34.2											
10	18-20	1.25		Brown, Clay, Stiff, w/ferrous stains and blocky structure	CH	37.6										0.60	
11	20-22	1.00		Brown, Clay, Stiff, w/silt seams, slickensided	CH	44.3	76.8	110.8	69	28	100.0	100.0	10.0	100.0	100.0	0.50	0.41
12	22-24	0.75		Brown, Clay, Medium stiff, w/blocky structure, slickensided	CH	34.6										0.35	
13	24-26	0.75		Brown, Clay, Medium stiff, w/blocky structure, slickensided	CH	39.5										0.35	

S # : Sample Number, P P : Pocket Penetrometer Reading, U S C : Unified Soil Classification, M c : Moisture Content

q u : Unconfined Compressive Strength, W O H : Weight of hammer, W O P : Weight of pipe

JOB NO. 14G533

DATE 1/29/92

PROJECT Gulf Intracoastal Waterway near Sargen Beach, Texas

BORING NO. 91-285

SAMPLE NO. 4

DEPTH 6-8 ft

SPECIMEN NO. 1

CLASSIFICATION

Gray&yellowish brown,Clay,Medium stiff,w/fe st,slickensided

Tare No.	P-323	Height	5.595 in.
Tare plus Wet Specimen	412.23 gm	Average Diameter	2.830 in.
Tare plus Dry Specimen	296.62 gm	Initial Area	6.290 sq in.
Water Weight	115.61 gm	Volume	35.194 cu in.
Tare Weight	42.65 gm	Volume of Solids	cu in.
Wet Specimen	1006.95 gm	Void Ratio	
Dry Specimen	691.96 gm	Saturation	%
Water Content	45.52 %	Dry Density	74.9 lb/cu ft
Specific Gravity of Solids			
LL =	PL =	PI =	

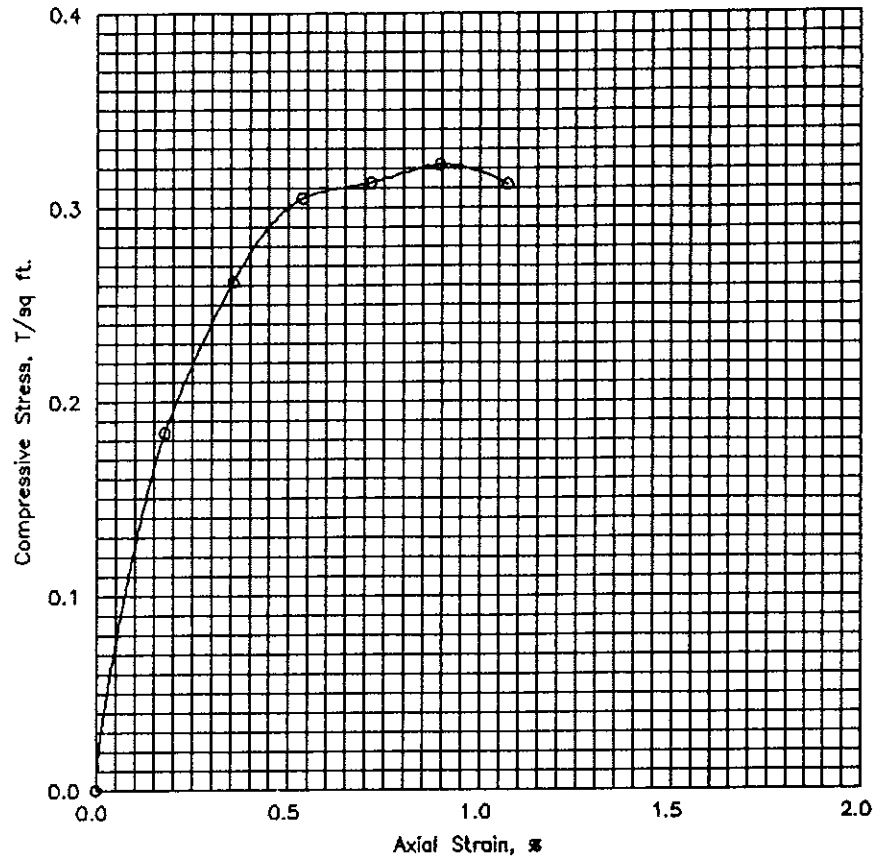
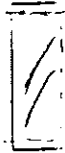
Proving Ring No. 10170

Proving Ring Constant, K = .766 lbs/div.

Elapsed Time min.	Dial Reading 0.001"	Cumulative Change in.	Proving Ring Dial Reading	Axial Load lb	Axial Strain	Area Corr. sq in.	Compr. Stress tsf
.0	0.	.000	.0	.0	.000	6.29	.000
.2	10.	.010	21.0	16.1	.002	6.30	.184
.3	20.	.020	30.0	23.0	.004	6.31	.262
.5	30.	.030	35.0	26.8	.005	6.32	.305
.7	40.	.040	36.0	27.6	.007	6.34	.313
.9	50.	.050	37.0	28.3	.009	6.35	.322
1.0	60.	.060	36.0	27.6	.011	6.36	.312

Job No. 14G533

Failure Sketches



- ☐ Controlled stress
☒ Controlled strain

Test No.	1			
Type of Specimen	Undisturbed			
Initial	Water content	w_0	45.5 %	
	Void ratio	e_0		
	Saturation	S_0	%	
	Dry density, lb/cu ft	γ_s	74.9	
	Time to failure, min	t_f	.87	
	Unconfined compressive strength, T/sq ft	q_u	.32	
	Undrained shear strength, T/sq ft	S_u	.16	
	Sensitivity ratio	S_t		
	Initial specimen diameter, in.	D_0	2.830	
	Initial specimen height, in.	H_0	5.595	

Classification Gray&yellowish brown,Clay,Medium stiff,w/ferrous stains,slickensided

LL PL PI G_s

Remarks _____

Project Gulf Intracoastal Waterway
Area Near Sargent Beach, Texas
Boring No. 91-285 Sample No. 4
Depth 6-8 ft Date 1/29/92
El _____

UNCONFINED COMPRESSION TEST REPORT

JOB NO. 14G533

DATE 1/29/92

PROJECT Gulf Intracoastal Waterway near Sargent Beach, Texas

BORING NO. 91-285

SAMPLE NO. 6

DEPTH 10-12 ft

SPECIMEN NO. 1

CLASSIFICATION

Brown, Clay, Medium stiff, Silty, w/silt pockets & blocky structure

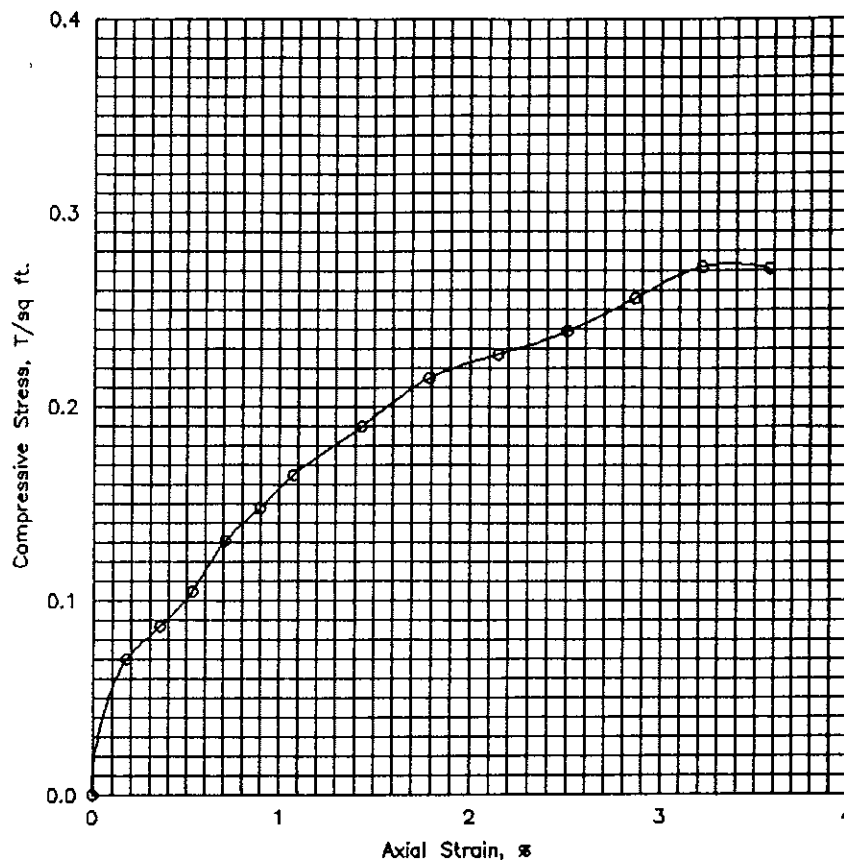
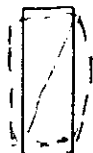
Tare No.	P-229	Height	5.595 in.
Tare plus Wet Specimen	442.74 gm	Average Diameter	2.830 in.
Tare plus Dry Specimen	343.10 gm	Initial Area	6.290 sq in.
Water Weight	99.64 gm	Volume	35.194 cu in.
Tare Weight	42.48 gm	Volume of Solids	cu in.
Wet Specimen	1083.64 gm	Void Ratio	
Dry Specimen	813.88 gm	Saturation	%
Water Content	33.14 %	Dry Density	88.1 lb/cu ft
Specific Gravity of Solids			
LL = 43	PL = 19	PI = 24	

Proving Ring No. 10170

Proving Ring Constant, K = .766 lbs/div.

Elapsed Time min.	Dial Reading 0.001"	Cumulative Change in.	Proving Ring Dial Reading	Axial Load lb	Axial Strain	Area Corr. sq in.	Compr. Stress tsf
.0	0.	.000	.0	.0	.000	6.29	.000
.2	10.	.010	8.0	6.1	.002	6.30	.070
.3	20.	.020	10.0	7.7	.004	6.31	.087
.5	30.	.030	12.0	9.2	.005	6.32	.105
.7	40.	.040	15.0	11.5	.007	6.34	.131
.9	50.	.050	17.0	13.0	.009	6.35	.148
1.0	60.	.060	19.0	14.6	.011	6.36	.165
1.4	80.	.080	22.0	16.9	.014	6.38	.190
1.6	100.	.100	25.0	19.1	.018	6.40	.215
2.0	120.	.120	26.5	20.3	.021	6.43	.227
2.3	140.	.140	28.0	21.4	.025	6.45	.239
2.7	160.	.160	30.0	23.0	.029	6.48	.256
3.0	180.	.180	32.0	24.5	.032	6.50	.272
3.3	200.	.200	32.0	24.5	.036	6.52	.271

Failure Sketches



- ☐ Controlled stress
☒ Controlled strain

Test No.		1			
Type of Specimen		Undisturbed			
Initial	Water content	w_0	33.1 %	%	%
	Void ratio	e_0			
	Saturation	S_0	%	%	%
	Dry density, lb/cu ft	γ_d	88.1		
Time to failure, min		t_f	3.02		
Unconfined compressive strength, T/sq ft		q_u	.27		
Undrained shear strength, T/sq ft		S_u	.14		
Sensitivity ratio		S_t			
Initial specimen diameter, in.		D_0	2.830		
Initial specimen height, in.		H_0	5.595		
Classification Brown, Clay, Medium stiff, Silty, w/silt pockets & blocky structure					
LL	43	PL	19	PI	24
Remarks		Project Gulf Intracoastal Waterway Area Near Sargent Beach, Texas Boring No. 91-285 Sample No. 6 Depth 10-12 ft Date 1/29/92 EI			
UNCONFINED COMPRESSION TEST REPORT					

JOB NO. 14G533

DATE 1/29/92

PROJECT Gulf Intracoastal Waterway near Sargent Beach, Texas

BORING NO. 91-285

SAMPLE NO. 11

DEPTH 20-22 ft

SPECIMEN NO. 1

CLASSIFICATION

Brown, Clay, Stiff, w/silt seams, slickensided

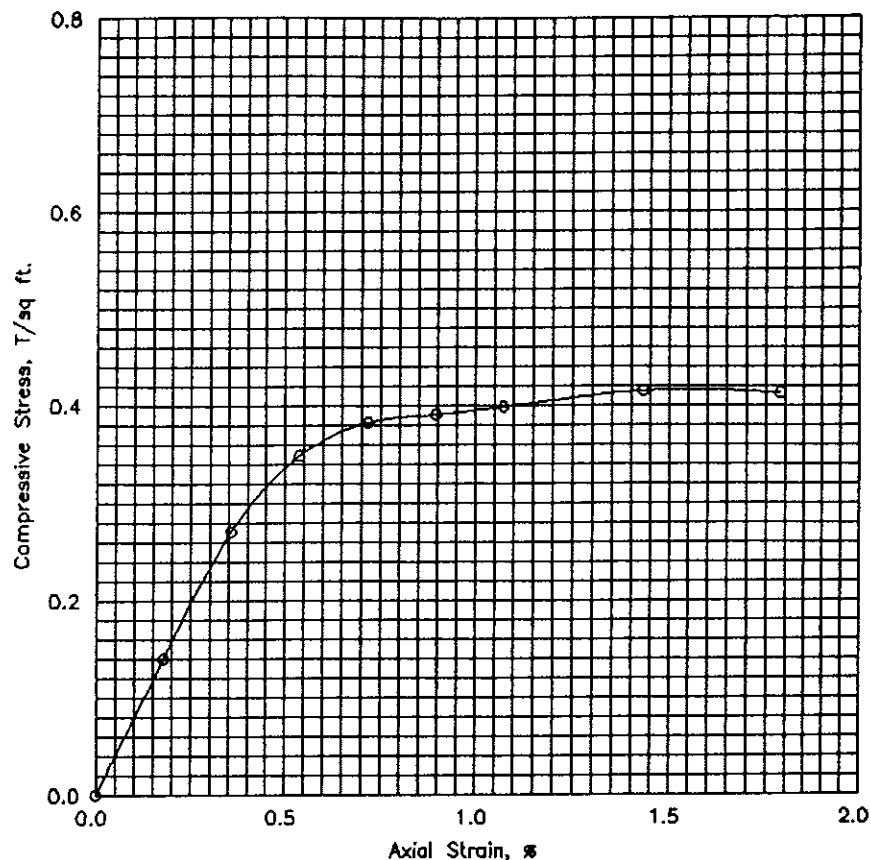
Tare No.	P-228	Height	5.595 in.
Tare plus Wet Specimen	450.28 gm	Average Diameter	2.830 in.
Tare plus Dry Specimen	325.14 gm	Initial Area	6.290 sq in.
Water Weight	125.14 gm	Volume	35.194 cu in.
Tare Weight	42.48 gm	Volume of Solids	cu in.
Wet Specimen	1023.95 gm	Void Ratio	
Dry Specimen	709.73 gm	Saturation	%
Water Content	44.27 %	Dry Density	76.8 lb/cu ft
Specific Gravity of Solids			
LL = 69	PL = 28	PI = 41	

Proving Ring No. 10170

Proving Ring Constant, K = .766 lbs/div.

Elapsed Time min.	Dial Reading 0.001"	Cumulative Change in.	Proving Ring Dial Reading	Axial Load lb	Axial Strain	Area Corr. sq in.	Compr. Stress tsf
.0	0.	.000	.0	.0	.000	6.29	.000
.2	10.	.010	16.0	12.3	.002	6.30	.140
.4	20.	.020	31.0	23.7	.004	6.31	.271
.6	30.	.030	40.0	30.6	.005	6.32	.349
.8	40.	.040	44.0	33.7	.007	6.34	.383
.9	50.	.050	45.0	34.5	.009	6.35	.391
1.1	60.	.060	46.0	35.2	.011	6.36	.399
1.4	80.	.080	48.0	36.8	.014	6.38	.415
1.7	100.	.100	48.0	36.8	.018	6.40	.413

Failure Sketches



- ☐ Controlled stress
☒ Controlled strain

Test No.		1			
Type of Specimen		Undisturbed			
Initial	Water content	w_0	44.3 %	%	%
	Void ratio	e_0			
	Saturation	S_0	%	%	%
	Dry density, lb/cu ft	γ_d	76.8		
Time to failure, min		t_f	1.42		
Unconfined compressive strength, T/sq ft		q_u	.41		
Undrained shear strength, T/sq ft		S_u	.21		
Sensitivity ratio		S_t			
Initial specimen diameter, in.		D_0	2.830		
Initial specimen height, in.		H_0	5.595		
Classification Brown, Clay, Stiff, w/silt seams, slickensided					
LL	69	PL	28	PI	41
Remarks		Project Gulf Intracoastal Waterway Area Near Sargent Beach, Texas Boring No. 91-285 Sample No. 11 Depth 20-22 ft Date 1/29/92 El UNCONFINED COMPRESSION TEST REPORT			