

LOG OF BORING AND TEST RESULTS

Boring: B-8

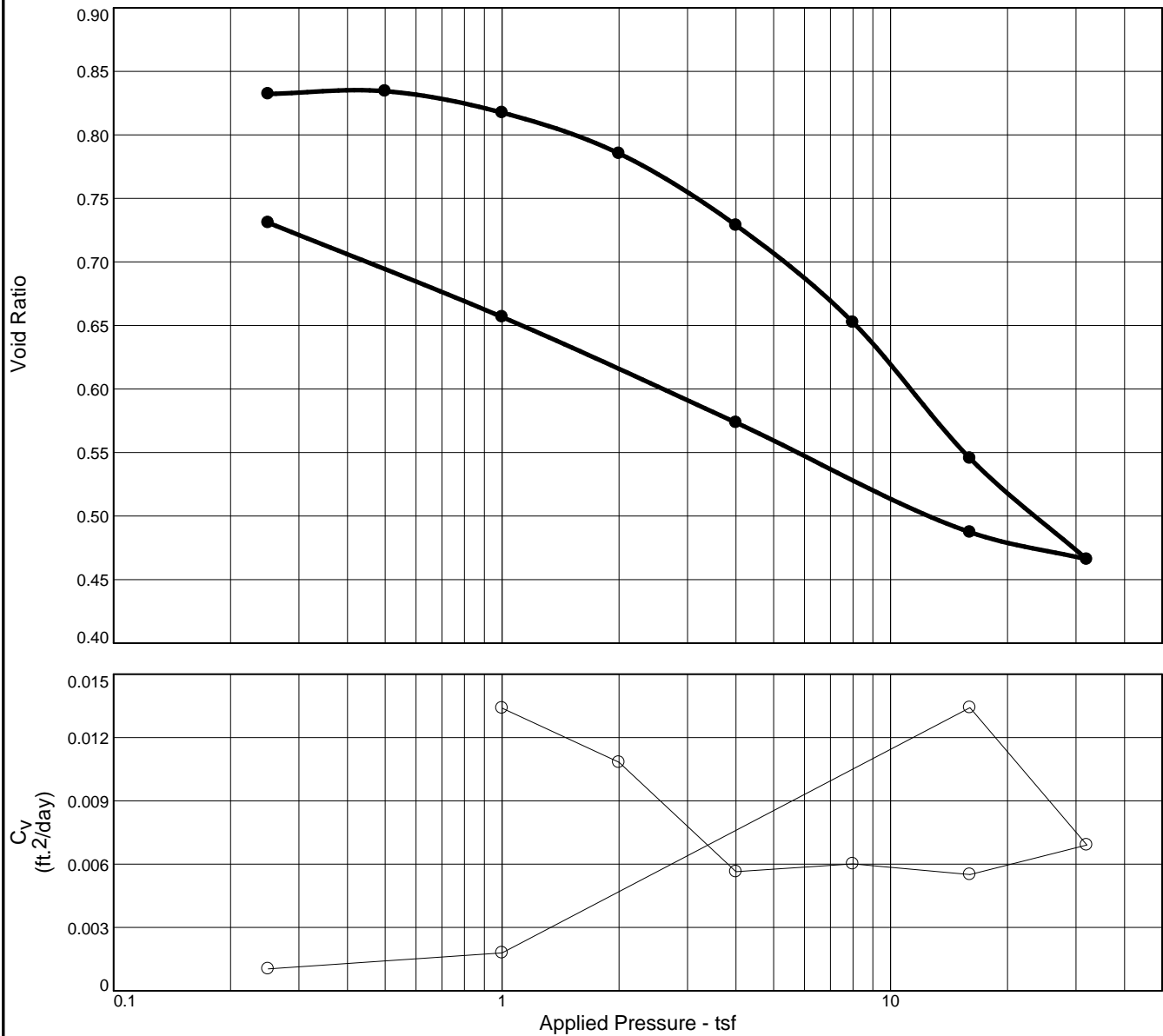
Project No: H0048
Date: 07/17/2022
Latitude: 29.30950°
Longitude: -94.96556°


Water Depth: See Text
Total Depth: 40.0 ft

| Scale in Feet | PP | SPT | SPLR | Symbol | Visual Classification | USC | Sample Number | Depth in Feet | Water Content % | Density | | Shear Tests | | | Atterberg Limits | | | Other Tests |
|---------------|------|-----|------|--------|--|-----|---------------|---------------|-----------------|---------|---------|-------------|---|-------|------------------|----|----|-------------|
| | | | | | | | | | | Dry pcf | Wet pcf | Type | φ | C psf | LL | PL | PI | |
| 0 | | | | | Moist, medium stiff to stiff gray & tan FAT CLAY w/trace of fine sand pockets | CH | 1A | 0 | 36 | | | | | | | | | |
| | 0.25 | | | | | | 1B | 1 | 29 | 92 | 119 | OB | 0 | 565 | | | | |
| | | | | | Moist, stiff gray & tan LEAN CLAY | CH | 2A | 2 | 27 | | | | | | | | | |
| | 0.50 | | | | Moist, soft gray & tan FAT CLAY w/few fine sand | CL | 2B | 3 | 31 | 92 | 120 | OB | 0 | 250 | | | | |
| 5 | 1.00 | | | | Moist, stiff gray & tan FAT CLAY w/trace of gravel | CH | 3A | 4 | 30 | | | | | | 66 | 17 | 49 | |
| | | | | | w/organic matter | | 3B | 5 | 22 | | | | | | | | | |
| | 1.00 | | | | | | 4A | 6 | 29 | | | | | | | | | |
| | | | | | Moist, stiff greenish-gray & gray LEAN CLAY w/trace of fine gravel | CL | 4B | 7 | 24 | | | | | | | | | |
| | 1.00 | | | | | | 5A | 8 | 22 | | | | | | | | | |
| 10 | 1.00 | | | | Moist, medium stiff gray & tan FAT CLAY | CH | 5B | 9 | 29 | 95 | 122 | OB | 0 | 739 | | | | |
| | | | | | Moist, stiff red, gray, & tan FAT CLAY w/trace of fine gravel & organic matter | CH | 6A | 10 | 27 | | | | | | | | | |
| | 1.00 | | | | | | 6B | 11 | 33 | | | | | | | | | |
| | | | | | | | 7A | 12 | 33 | | | | | | | | | |
| | 1.00 | | | | | | 7B | 13 | 29 | | | | | | | | | |
| 15 | 1.00 | | | | Moist, medium stiff reddish-brown & gray FAT CLAY w/trace of concretions | CH | 8A | 14 | 29 | | | | | | | | | |
| | | | | | Moist, stiff reddish-brown & gray LEAN CLAY | CL | 8B | 15 | 32 | 89 | 117 | OB | 0 | 833 | | | | |
| | 1.00 | | | | | | 9A | 16 | 23 | | | | | | 35 | 14 | 21 | |
| | | | | | | | 9B | 17 | 23 | | | | | | | | | |
| 20 | 1.00 | | | | Moist, soft reddish-brown SILT W/SAND (fine) | ML | 10A | 18 | 33 | | | | | | | | | |
| | | | | | | | 10B | 19 | 21 | | | | | | | | | |
| | | | | | Moist, very stiff red & gray LEAN CLAY | CL | 11A | 23 | 26 | | | | | | | | | |
| 25 | 1.00 | | | | Moist, soft reddish-brown LEAN CLAY | CL | 11B | 24 | 19 | | | | | | | | | |
| | | | | | | | | | | | | | | | | | | |
| | | | | | | | 12A | 28 | 27 | | | | | | | | | |
| 30 | 1.00 | | | | | | 12B | 29 | 24 | | | | | | 75 | 24 | 51 | CON |
| | | | | | | | | | | | | | | | | | | |
| | | | | | Moist, stiff reddish-brown FAT CLAY | CH | 13A | 33 | 29 | | | | | | | | | |
| 35 | 1.00 | | | | | | 13B | 34 | 28 | | | | | | | | | |
| | | | | | | | | | | | | | | | | | | |
| | | | | | w/trace of fine sand pockets | | 14A | 38 | 28 | | | | | | | | | |
| 40 | 1.00 | | | | | | 14B | 39 | 30 | 93 | 120 | OB | 0 | 1005 | | | | |
| | | | | | | | | | | | | | | | | | | |
| 45 | | | | | | | | | | | | | | | | | | |
| 50 | | | | | | | | | | | | | | | | | | |

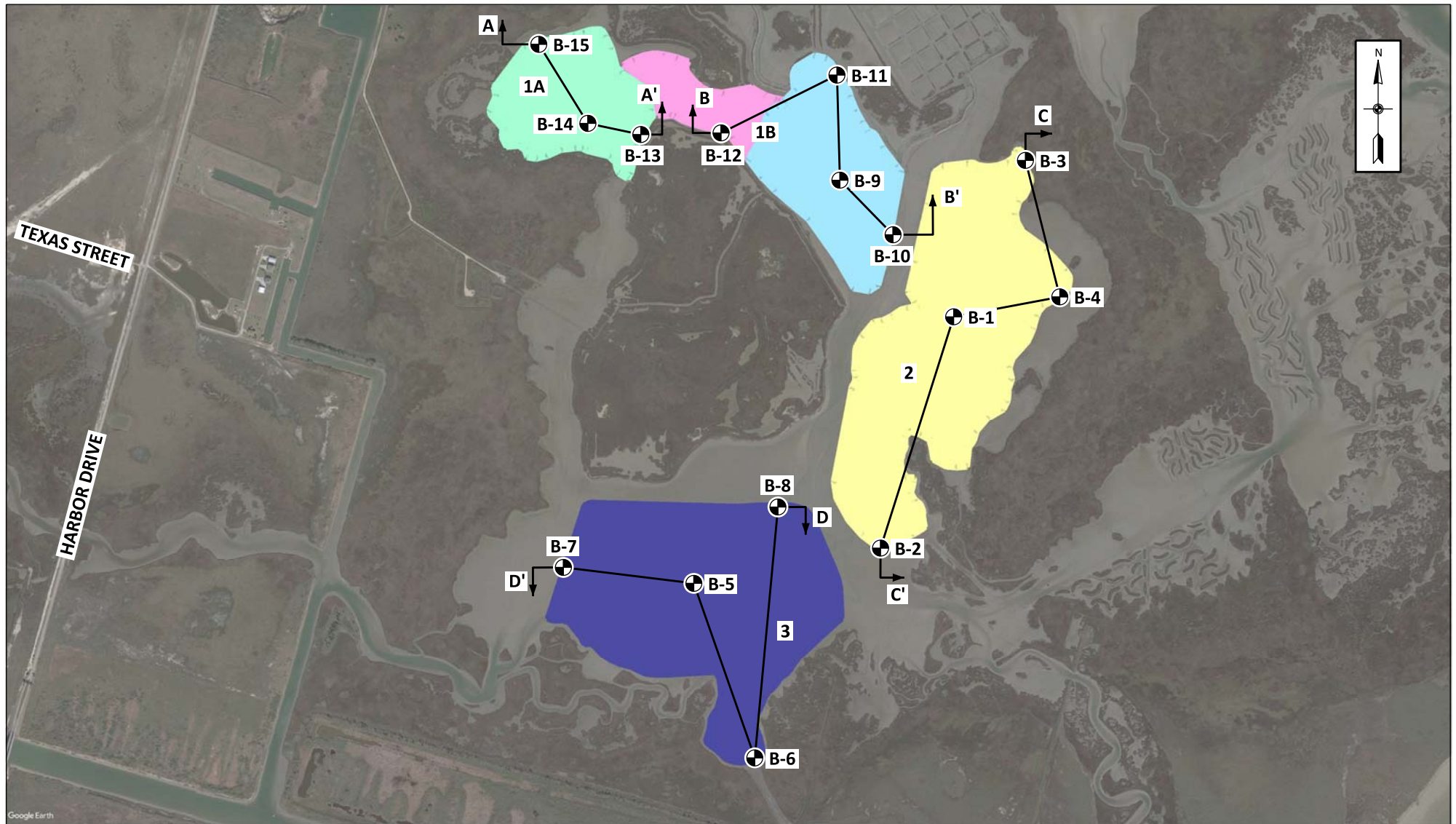
NOTES: Boring 8 was drilled in 2 ft. of water.

CONSOLIDATION TEST REPORT



| | | | | | | | | |
|---|----------|--|----|--------------------|---------|-------------------------|----------------|-----------------------|
| Natural | | Dry Dens. (pcf) | LL | PI | Sp. Gr. | P _c (tsf) | C _c | Initial Void Ratio |
| Saturation | Moisture | | | | | | | |
| 99.5 % | 30.2 % | | | | | | | |
| M, so r-br FT CL | | | | | | | | |
| MATERIAL DESCRIPTION | | | | | | | USCS | AASHTO |
| | | | | | | | CH | |
| Project No. H0048 | | Client: DUCKS UNLIMITED, INC., RICHMOND, TEXAS | | | | | Remarks: | |
| Project: DUCKS UNLIMITED, INC. - PIERCE MARSH BENEFICIAL USE MARSH CREATION, PHASES 1 AND 2, | | | | | | | | |
| Source of Sample: B-8 | | Depth: 27.5 | | Sample Number: 12B | | | | |
|  EUSTIS ENGINEERING SINCE 1946 | | | | | | | Figure | |

Tested By: BH _____ Checked By: RR _____



SATELLITE IMAGERY DATED: JANUARY 2022

NOT TO SCALE

⊕ DENOTES APPROXIMATE LOCATIONS OF SOIL BORINGS DRILLED BETWEEN 11 AND 18 JULY 2022

BORING LOCATION PLAN

PHASE 1
DUCKS UNLIMITED, INC.
PIERCE MARSH BENEFICIAL USE MARSH CREATION
NORTH OF WEST BAY NEAR GALVESTON ISLAND
GALVESTON COUNTY, TEXAS
DU CONTRACT NO. TX-0-2
DU PROJECT NO. TX-194-4
DU TASK ORDER NO. 1









| | |
|---------------------------------|-------------------|
| DRAWN BY: S.T.S. | JOB NO.: H0048 |
| CHECKED BY: H.C.W. | DATE: 15 AUG 2022 |
| CADD FILE: LOCATION PLAN.DGN | FIGURE 2 |



PP Pocket penetrometer: Resistance in tons per square foot

SPT Standard Penetration Test: Number of blows of a 140-lb hammer dropped 30 inches required to drive 2-in. O.D., 1.4-in. I.D. sampler a distance of 1 foot into the soil after first seating it 6 inches. Values shown have not been corrected.

SPLR Type of Sampling  Shelby  SPT  Auger  Vibracore  Geoprobe  No sample

SYMBOL Clay  Silt  Sand  Peat/Humus  Shells  Stone/Gravel 
Predominant type shown heavy; modifying type shown light

USC Unified Soil Classification

DENSITY Unit weight in pounds per cubic foot

SHEAR TESTS

TYPE

UC Unconfined compression shear

OB Unconsolidated undrained triaxial compression shear on one specimen confined at the approximate overburden pressure

UU Unconsolidated undrained triaxial compression shear

ϕ Angle of internal friction in degrees

c Cohesion in pounds per square foot

ATTERBERG LIMITS

LL Liquid Limit

PL Plastic Limit

PI Plasticity Index

OTHER TESTS

CON Consolidation

-#200 Percent passing a U.S. No. 200 sieve

SV Particle size distribution (sieve only)

PD Particle size distribution (sieve and hydrometer)

k Coefficient of permeability in centimeters per second

SP Swelling pressure in pounds per square foot

Other laboratory test results reported on separate figures

GENERAL NOTES

- (1) If a ground water depth is shown on the boring log, these observations were made at the time of drilling and were measured below the existing ground surface. These observations are shown on the boring logs. However, ground water levels may vary due to seasonal fluctuations and other factors. If important to construction, the depth to ground water should be determined by those persons responsible for construction immediately prior to beginning work.
- (2) While the individual logs of borings are considered to be representative of subsurface conditions at their respective locations on the dates shown, it is not warranted that they are representative of subsurface conditions at other locations and times.