

U.S. ARMY CORPS OF ENGINEERS

DEPTH, FEET		SAMPLE NO.	PEN./TORVANE	SPT.-BLOW COUNT	BORING NO. <u>94-15</u>	DATE: BEGIN <u>3/16/94</u>	PAGE <u>1/2</u>
					JOB NO. <u>146727-B</u>	COMPLETE <u>3/16/94</u>	Thin Walled Tube
					PROJECT <u>Wallisville Lake, Trinity River & Tributaries</u>	☒ 3"	☐ 6"
					LOCATION <u>Near Wallisville, TX</u>		
					ELEVATION OF HOLE _____		
					MANUFACTURER'S DESIGNATION OF DRILL RIG <u>Fairbanks 3b</u>		
					GROUNDWATER: DEPTH <u>0</u> ft., ELEV. _____ ft., at end of Drilling		
					WEATHER <u>SUNNY, WARM</u>		
					DRILLER <u>RANDY GEARIN</u>	LOGGER <u>Peter Kosowski</u>	
					COLOR	MATERIAL TYPE	CONSISTENCY
					SECONDARY CONSTITUENTS		STRUCTURAL FEATURES AND COMMENTS
0		1	0.75	Dark Gray	clay	m. stiff	
		2	2.0	Dark Gray	clay	v. stiff	shale nodules
5		3	2.0	Yellowish Brown	clay	v. stiff	
		4	2.5	Yellowish Brown	clay	v. stiff	shale nodules, trace sand oxide nodules, Fe stains
		5	4.5	Yellowish Brown	clay	Hard	
10		6	4.5	Yellowish Brown	clay	Hard	blocky structure, silts 10'-12'
		7	4.5	Yellowish Brown	clay	Hard	shale nodules sils 14'-16'
15		8	4.5	Yellowish Brown	clay	Hard	sils 16'-18'
		9	4.5	Yellowish Brown	clay	Hard	encounter water @ 20'
20		10	2.5	Yellowish Brown	clay	v. stiff	sils 20'-22'
		11	4.5	Yellowish Brown	clay	Hard	shale nodules
		12	4.5	Yellowish Brown	clay	Hard	shale nodules
25		13	2.75	Yellowish Brown	clay	v. stiff	shale nodules
		14	2.75	Yellowish Brown	clay	v. stiff	shale nodules
		15	3.0	Yellowish Brown	clay	v. stiff	shale nodules
30		16	4.0	Yellowish Brown	clay	very stiff	shale nodules
		17	3.0	Yellowish Brown	clay	v. stiff	sand, shale nodules
35		18	2.25	Reddish Brown	clay	v. stiff	sand

U.S. ARMY CORPS OF ENGINEERS

DEPTH, FEET		SAMPLE NO.	PEN./TORVANE	SPT.-BLOW COUNT	BORING NO. <u>9A-15</u> DATE: BEGIN <u>3/16/94</u> PAGE <u>2</u> / <u>12</u> JOB NO. <u>146727-6</u> COMPLETE <u>3/16/94</u> Thin Walled Tube PROJECT <u>Wallisville Lake, Trinity River & Tributaries</u> ☒ 3" ☐ 6" LOCATION <u>near Wallisville, TX</u> ELEVATION OF HOLE _____ MANUFACTURER'S DESIGNATION OF DRILL RIG <u>FALING 36</u> GROUNDWATER: DEPTH <u>0</u> ft., ELEV. _____ ft., at end of Drilling WEATHER <u>Cloudy, warm</u> DRILLER <u>PAUL GEORGE</u> LOGGER <u>PETER KOMOSKI</u>				
					COLOR	MATERIAL TYPE	CONSISTENCY	SECONDARY CONSTITUENTS	STRUCTURAL FEATURES AND COMMENTS
35		19	4.0	Reddish Brown	clay	v. stiff		s/s	
40		20	3.75	Reddish Brown	clay	v. stiff		s/s	
45								** water level at end of Drilling	
50									
55									
60									
65									
70									

JOB NO. 14G727-12

DATE 4/12/94

PROJECT Lake Wallisville, Trinity River
and Tributaries,

AREA near Wallisville, Texas

BORING NO. 94-15

SAMPLE NO. 3

DEPTH 4-6 ft

SPECIMEN NO. 1

CLASSIFICATION

Gray, Clay, Very stiff

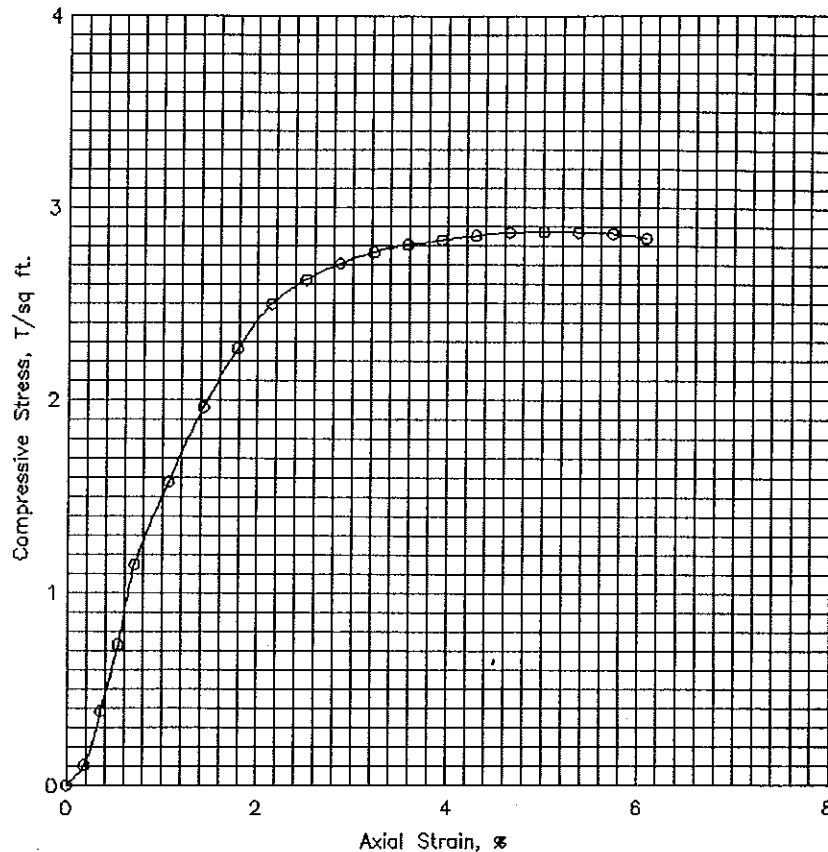
Tare No.	KL-24	Height	5.595 in.
Tare plus Wet Specimen	308.84 gm	Average Diameter	2.830 in.
Tare plus Dry Specimen	261.05 gm	Initial Area	6.290 sq in.
Water Weight	47.79 gm	Volume	35.194 cu in.
Tare Weight	42.59 gm	Volume of Solids	cu in.
Wet Specimen	1144.88 gm	Void Ratio	
Dry Specimen	939.38 gm	Saturation	%
Water Content	21.88 %	Dry Density	101.7 lb/cu ft
Specific Gravity of Solids			
LL = 58	PL =	PI =	

Proving Ring No. 10170

Proving Ring Constant, K = .766 lbs/div.

Elapsed Time min.	Dial Reading 0.001"	Cumulative Change in.	Proving Ring Dial Reading	Axial Load lb	Axial Strain	Area Corr. sq in.	Compr. Stress tsf
.0	0.	.000	.0	.0	.000	6.29	.000
.2	10.	.010	12.0	9.2	.002	6.30	.105
.3	20.	.020	44.0	33.7	.004	6.31	.384
.5	30.	.030	84.0	64.3	.005	6.32	.733
.7	40.	.040	132.0	101.1	.007	6.34	1.149
1.0	60.	.060	182.0	139.4	.011	6.36	1.579
1.4	80.	.080	227.0	173.9	.014	6.38	1.962
1.7	100.	.100	263.0	201.5	.018	6.40	2.265
2.0	120.	.120	291.0	222.9	.021	6.43	2.497
2.3	140.	.140	307.0	235.2	.025	6.45	2.624
2.7	160.	.160	318.0	243.6	.029	6.48	2.708
3.0	180.	.180	326.0	249.7	.032	6.50	2.766
3.3	200.	.200	332.0	254.3	.036	6.52	2.807
3.7	220.	.220	336.0	257.4	.039	6.55	2.830
4.0	240.	.240	340.0	260.4	.043	6.57	2.853
4.3	260.	.260	343.0	262.7	.046	6.60	2.868
4.7	280.	.280	345.0	264.3	.050	6.62	2.874
5.0	300.	.300	346.0	265.0	.054	6.65	2.871
5.3	320.	.320	346.5	265.4	.057	6.67	2.864
5.7	340.	.340	345.0	264.3	.061	6.70	2.841

Failure Sketches


☐ Controlled stress

☒ Controlled strain

Test No.		1			
Type of Specimen		Undisturbed			
Initial	Water content	w_0	21.9 %	%	%
	Void ratio	e_0			
	Saturation	S_0	%	%	%
	Dry density, lb/cu ft	γ_d	101.7		
Time to failure, min		t_f	4.67		
Unconfined compressive strength, T/sq ft		q_u	2.87		
Undrained shear strength, T/sq ft		S_u	1.44		
Sensitivity ratio		S_t			
Initial specimen diameter, in.		D_0	2.830		
Initial specimen height, in.		H_0	5.595		
Classification Gray, Clay, Very stiff					
LL	58	PL		PI	G_s
Remarks		Project Lake Wallisville, Trinity River			
		and Tributaries,			
		Area near Wallisville, Texas			
		Boring No. 94-15	Sample No. 3		
		Depth 4-6 ft	Date 4/12/94		
		UNCONFINED COMPRESSION TEST REPORT			

JOB NO. 14G727-12

DATE 4/12/94

PROJECT Lake Wallisville, Trinity River
and Tributaries,

AREA near Wallisville, Texas

BORING NO. 94-15

SAMPLE NO. 7

DEPTH 12-14 ft

SPECIMEN NO. 1

CLASSIFICATION

Yellowish gray, Clay, Very stiff, w/calcareous nodules & blocky structure

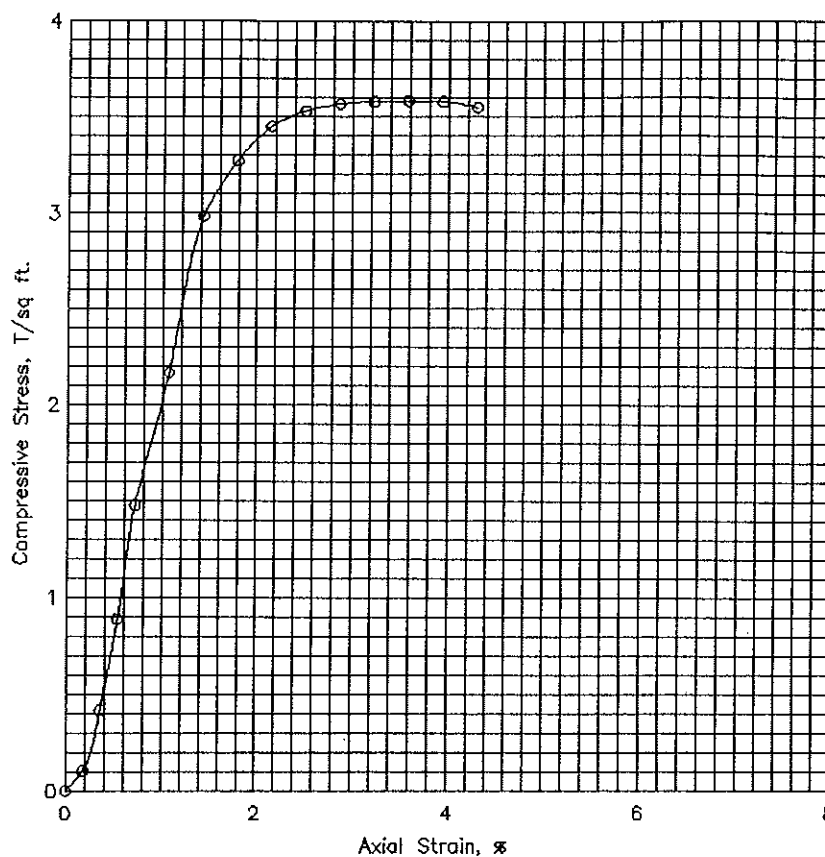
Tare No.	P-219	Height	5.595 in.
Tare plus Wet Specimen	379.80 gm	Average Diameter	2.830 in.
Tare plus Dry Specimen	304.33 gm	Initial Area	6.290 sq in.
Water Weight	75.47 gm	Volume	35.194 cu in.
Tare Weight	42.64 gm	Volume of Solids	cu in.
Wet Specimen	1145.05 gm	Void Ratio	
Dry Specimen	888.74 gm	Saturation	%
Water Content	28.84 %	Dry Density	96.2 lb/cu ft
Specific Gravity of Solids			
LL = 79	PL =	PI =	

Proving Ring No. 10170

Proving Ring Constant, K = .766 lbs/div.

Elapsed Time min.	Dial Reading 0.001"	Cumulative Change in.	Proving Ring Dial Reading	Axial Load lb	Axial Strain	Area Corr. sq in.	Compr. Stress tsf
.0	0.	.000	.0	.0	.000	6.29	.000
.2	10.	.010	12.0	9.2	.002	6.30	.105
.4	20.	.020	48.0	36.8	.004	6.31	.419
.5	30.	.030	102.0	78.1	.005	6.32	.890
.7	40.	.040	170.0	130.2	.007	6.34	1.480
1.0	60.	.060	250.0	191.5	.011	6.36	2.168
1.4	80.	.080	345.0	264.3	.014	6.38	2.982
1.7	100.	.100	380.0	291.1	.018	6.40	3.272
2.1	120.	.120	402.0	307.9	.021	6.43	3.449
2.4	140.	.140	413.0	316.4	.025	6.45	3.531
2.7	160.	.160	419.0	321.0	.029	6.48	3.569
3.0	180.	.180	422.0	323.3	.032	6.50	3.581
3.4	200.	.200	424.0	324.8	.036	6.52	3.585
3.7	220.	.220	425.0	325.5	.039	6.55	3.580
4.1	240.	.240	423.0	324.0	.043	6.57	3.550

Failure Sketches



☐ Controlled stress
☒ Controlled strain

Test No.		1			
Type of Specimen		Undisturbed			
Initial	Water content	w_0	28.8 %	%	%
	Void ratio	e_0			
	Saturation	S_0	%	%	%
	Dry density, lb/cu ft	γ_d	96.2		
Time to failure, min		t_f	3.37		
Unconfined compressive strength, T/sq ft		q_u	3.58		
Undrained shear strength, T/sq ft		S_u	1.79		
Sensitivity ratio		S_t			
Initial specimen diameter, in.		D_0	2.830		
Initial specimen height, in.		H_0	5.595		
Classification Yellowish gray, Clay, Very stiff, w/calcareous nodules & blocky structure					
LL	79	PL		PI	C_u
Remarks		Project Lake Wallisville, Trinity River			
		and Tributaries,			
		Area near Wallisville, Texas			
		Boring No. 94-15		Sample No. 7	
		Depth 12-14 ft		Date 4/12/94	
		UNCONFINED COMPRESSION TEST REPORT			

JOB NO. 14G727-12

DATE 4/12/94

PROJECT Lake Wallisville, Trinity River
and Tributaries,

AREA near Wallisville, Texas

BORING NO. 94-15

SAMPLE NO. 10

DEPTH 18-20 ft

SPECIMEN NO. 1

CLASSIFICATION

Yellowish gray, Clay, Hard, w/calcareous nodules&blocky structure

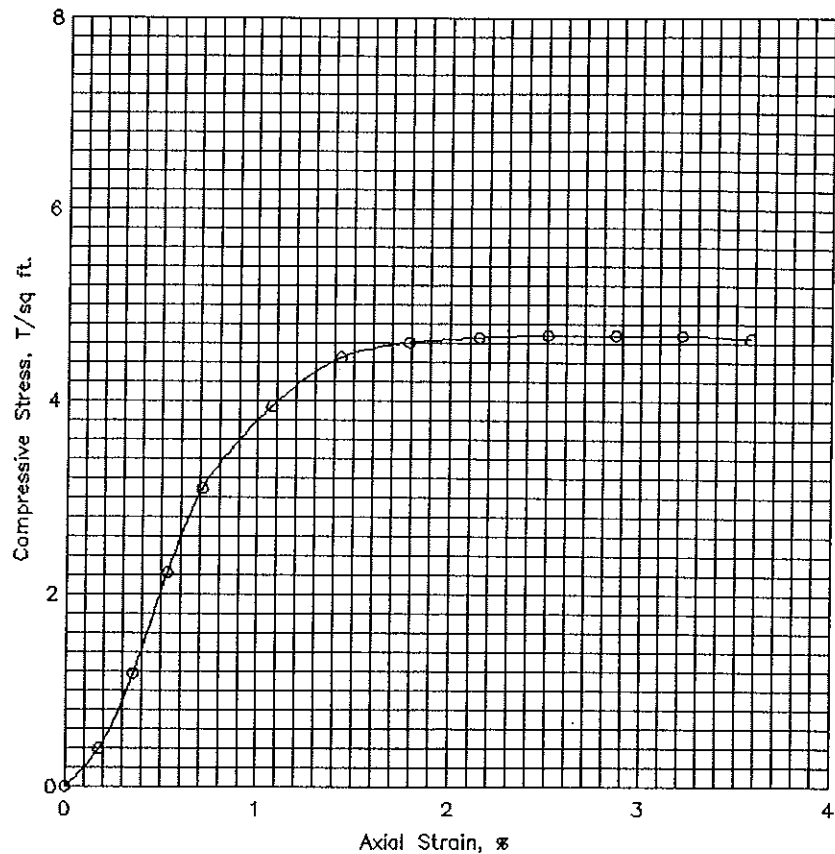
Tare No.	K1-32	Height	5.595 in.
Tare plus Wet Specimen	396.78 gm	Average Diameter	2.830 in.
Tare plus Dry Specimen	328.13 gm	Initial Area	6.290 sq in.
Water Weight	68.65 gm	Volume	35.194 cu in.
Tare Weight	42.59 gm	Volume of Solids	cu in.
Wet Specimen	1185.76 gm	Void Ratio	
Dry Specimen	955.93 gm	Saturation	%
Water Content	24.04 %	Dry Density	103.5 lb/cu ft
Specific Gravity of Solids			
LL = 74	PL =	PI =	

Proving Ring No. 10170

Proving Ring Constant, K = .766 lbs/div.

Elapsed Time min.	Dial Reading 0.001"	Cumulative Change in.	Proving Ring Dial Reading	Axial Load lb	Axial Strain	Area Corr. sq in.	Compr. Stress tsf
.0	0.	.000	.0	.0	.000	6.29	.000
.2	10.	.010	45.0	34.5	.002	6.30	.394
.3	20.	.020	135.0	103.4	.004	6.31	1.179
.5	30.	.030	255.0	195.3	.005	6.32	2.224
.7	40.	.040	355.0	271.9	.007	6.34	3.090
1.1	60.	.060	455.0	348.5	.011	6.36	3.947
1.4	80.	.080	515.0	394.5	.014	6.38	4.451
1.7	100.	.100	535.0	409.8	.018	6.40	4.607
2.0	120.	.120	543.0	415.9	.021	6.43	4.659
2.4	140.	.140	548.0	419.8	.025	6.45	4.685
2.7	160.	.160	550.0	421.3	.029	6.48	4.684
3.0	180.	.180	552.0	422.8	.032	6.50	4.684
3.3	200.	.200	550.0	421.3	.036	6.52	4.650

Failure Sketches



☐ Controlled stress
☒ Controlled strain

Test No.		1			
Type of Specimen		Undisturbed			
Initial	Water content	w_0	24.0 %	%	%
	Void ratio	e_0			
	Saturation	S_0	%	%	%
	Dry density, lb/cu ft	γ_d	103.5		
Time to failure, min		t_f	2.37		
Unconfined compressive strength, T/sq ft		q_u	4.68		
Undrained shear strength, T/sq ft		S_u	2.34		
Sensitivity ratio		S_t			
Initial specimen diameter, in.		D_0	2.830		
Initial specimen height, in.		H_0	5.595		
Classification Yellowish gray, Clay, Hard, w/calcareous nodules & blocky structure					
LL	74	PL		PI	G_s
Remarks		Project Lake Wallisville, Trinity River			
		and Tributaries,			
		Area near Wallisville, Texas			
		Boring No. 94-15		Sample No. 10	
		Depth 18-20 ft		Date 4/12/94	
UNCONFINED COMPRESSION TEST REPORT					

JOB NO. 14G727-12

DATE 4/12/94

PROJECT Lake Wallisville, Trinity River
and Tributaries,

AREA near Wallisville, Texas

BORING NO. 94-15

SAMPLE NO. 13

DEPTH 24-26 ft

SPECIMEN NO. 1

CLASSIFICATION

Yellowish gray, Clay, Very stiff, w/calcareous nodules&sand parting

Tare No.	W-19	Height	5.595 in.
Tare plus Wet Specimen	453.16 gm	Average Diameter	2.830 in.
Tare plus Dry Specimen	377.20 gm	Initial Area	6.290 sq in.
Water Weight	75.96 gm	Volume	35.194 cu in.
Tare Weight	42.66 gm	Volume of Solids	cu in.
Wet Specimen	1182.40 gm	Void Ratio	
Dry Specimen	963.61 gm	Saturation	%
Water Content	22.71 %	Dry Density	104.3 lb/cu ft
Specific Gravity of Solids			
LL = 57	PL =	PI =	

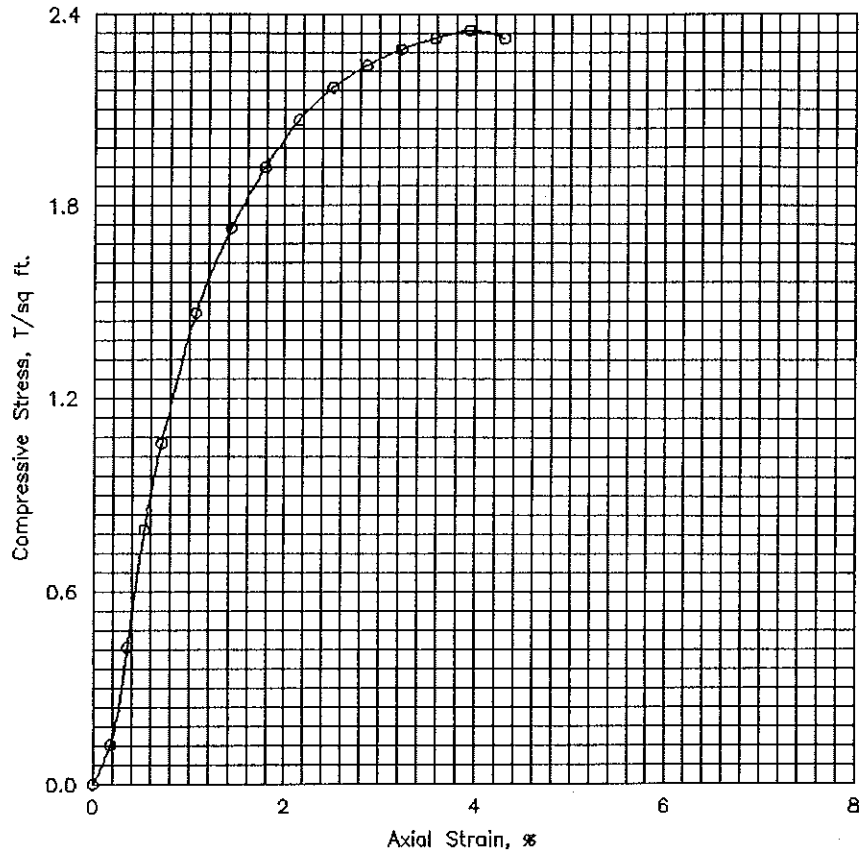
Proving Ring No. 10170

Proving Ring Constant, K = .766 lbs/div.

Elapsed Time min.	Dial Reading 0.001"	Cumulative Change in.	Proving Ring Dial Reading	Axial Load lb	Axial Strain	Area Corr. sq in.	Compr. Stress tsf
.0	0.	.000	.0	.0	.000	6.29	.000
.2	10.	.010	14.0	10.7	.002	6.30	.123
.4	20.	.020	49.0	37.5	.004	6.31	.428
.6	30.	.030	91.0	69.7	.005	6.32	.794
.8	40.	.040	122.0	93.5	.007	6.34	1.062
1.2	60.	.060	169.0	129.5	.011	6.36	1.466
1.5	80.	.080	200.0	153.2	.014	6.38	1.729
1.9	100.	.100	223.0	170.8	.018	6.40	1.920
2.3	120.	.120	241.0	184.6	.021	6.43	2.068
2.6	140.	.140	254.0	194.6	.025	6.45	2.171
2.9	160.	.160	263.0	201.5	.029	6.48	2.240
3.2	180.	.180	270.0	206.8	.032	6.50	2.291
3.6	200.	.200	275.0	210.6	.036	6.52	2.325
3.9	220.	.220	279.0	213.7	.039	6.55	2.350
4.2	240.	.240	277.0	212.2	.043	6.57	2.325

Job No. 14G727-12

Failure Sketches



☐ Controlled stress
☒ Controlled strain

Test No.		1			
Type of Specimen		Undisturbed			
Initial	Water content	w_0	22.7 %	%	%
	Void ratio	e_0			
	Saturation	S_0	%	%	%
	Dry density, lb/cu ft	γ_d	104.3		
Time to failure, min		t_f	3.90		
Unconfined compressive strength, T/sq ft		q_u	2.35		
Undrained shear strength, T/sq ft		S_u	1.18		
Sensitivity ratio		S_t			
Initial specimen diameter, in.		D_0	2.830		
Initial specimen height, in.		H_0	5.595		
Classification Yellowish gray, Clay, Very stiff, w/calcareous nodules & sand parting					
LL	57	PL		PI	G_c
Remarks		Project Lake Wallisville, Trinity River			
		and Tributaries,			
		Area near Wallisville, Texas			
		Boring No. 94-15	Sample No. 13		
		Depth EI	24-26 ft	Date	4/12/94
UNCONFINED COMPRESSION TEST REPORT					

SUMMARY OF LABORATORY TEST RESULTS

Contract No. DACW64-93-D-0005 Delivery Order No. 0012

Boring No. 94-15

S #	Depth (ft)	P P (tsf)	SPT Blows per Foot	Visual Classification	U S C	M c (%)	Dry Unit Wt (pcf)	Wet Unit Wt (pcf)	L L (%)	P L (%)	Mechanical Analysis % Passing					Torvane Shear Strength (tsf)	q u (tsf)
											#4	#10	#40	#100	#200		
1	0 - 2	1.00		Dark gray, Clay, Stiff, w/grass roots	C H	24.6											
2	2 - 4	1.50		Gray, Clay, Very stiff	C H	23.7											
3	4 - 6	2.25		Gray, Clay, Very stiff	C H	21.9	101.7	123.9	58		100.0	100.0	100.0	99.0	98.7		2.87
4	6 - 8	2.50		Yellowish gray, Clay, Very stiff, w/sand parting	C H	20.5											
5	8 - 10	4.00		Yellowish gray, Clay, Very stiff, w/calcareous nodules&sand parting	C H	23.0											
6	10 - 12	2.50		Yellowish gray, Clay, Very stiff, w/calcareous nodules&blocky structure	C H	26.5											
7	12 - 14	4.00		Yellowish gray, Clay, Very Stiff, w/calcareous nodules and blocky structure	C H	28.8	96.2	123.9	79		100.0	100.0	99.7	98.6	98.4		3.58
8	14 - 16	4.50		Yellowish gray, Clay, Hard, w/ferrous stains and blocky structure	C H	26.5											
9	16 - 18	4.50		Yellowish gray, Clay, Hard, w/blocky structure	C H	26.8											
10	18 - 20	4.50		Yellowish gray, Clay, Hard, w/calcareous nodules and blocky structure	C H	24.0	103.5	128.4	74		100.0	100.0	99.0	98.6	98.0		4.68
11	20 - 22	4.50		Yellowish gray, Clay, Hard, w/blocky structure	C H	23.7											
12	22 - 24	4.00		Yellowish gray, Clay, Very stiff, w/blocky structure	C H	22.8											
13	24 - 26	3.50		Yellowish gray, Clay, Very stiff, w/calcareous nodules and sand parting	C H	22.7	104.3	127.9	57		100.0	100.0	99.7	99.1	98.0		2.35
14	26 - 28	3.50		Yellowish gray, Clay, Very stiff, w/calcareous nodules and sand parting	C H	24.3											
15	28 - 30	4.50		Yellowish gray, Clay, Hard, w/sand parting and calcareous nodules	C H	24.1											
16	30 - 32	4.50		Yellowish gray, Clay, Hard, w/sand parting	C H	24.4											
17	32 - 34	2.00		Reddish brown&gray, Clay, Very stiff, Sandy, w/calcareous nodules	C L	25.8											
18	34 - 36	3.50		Reddish brown, Clay, Very stiff, Sandy, w/sand parting and ferrous stain	C L	23.4											
19	36 - 38	3.50		Reddish brown, Clay, Very stiff, w/silt parting	C H	26.9											
20	38 - 40	4.50		Reddish brown, Clay, Hard, w/blocky structure	C H	26.0											

S # : Sample Number, P P : Pocket Penetrometer Reading, U S C : Unified Soil Classification, M c : Moisture Content
q u : Uncogined Compressive Strength, W O H : Weight of hammer, W O P : Weight of Pipe